

SECTION 200 -- EARTHWORK

SECTION 201 -- CLEARING AND GRUBBING

201.01 Description. This work shall consist of clearing, grubbing, removing, and disposing of all vegetation and debris, except such objects as are designated to remain or are to be removed in accordance with other sections of these specifications, within the construction limits shown on the plans. If no construction limits are shown, the right-of-way and easement areas will be the construction limits. This work shall include the preservation from injury or defacement of all vegetation and objects designated to remain.

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CONSTRUCTION REQUIREMENTS

201.02 General. Right-of-way lines and construction lines will be established, and trees, shrubs, plants, and other things to remain will be designated. All such designated items shall be preserved. Any damage to natural terrain or to vegetation or objects designated to remain shall be repaired, replaced, or otherwise compensated for, as determined, with no additional payment. Tree wound dressing required for cut or scarred surfaces of trees or shrubs selected for retention shall be in accordance with 914.09(c).

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201.03 Clearing and Grubbing. Surface objects, trees, stumps, roots, and other protruding obstructions not designated to remain shall be cleared and grubbed, including mowing as required. Undisturbed sound stumps, roots, and nonperishable solid objects, which are a minimum of 1 m (3 ft) below the final subgrade or slope of embankments, may be left, provided they are as nearly flush as possible. However, they shall not extend more than 100 mm (4 in.) above the ground line or low water level. Sound stumps may be cut off at ground level outside the construction limits of cut and embankment areas if approved.

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Except in areas to be excavated, stump holes and other holes from which obstructions are removed shall be backfilled with suitable material and compacted in accordance with 203.23.

Burning of perishable material shall be done in accordance with applicable laws, ordinances, rules, and regulations. All necessary permits approval shall be obtained prior to burning.

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Unless burned in accordance with the requirements herein, perishable materials and debris shall be removed from the right-of-way and disposed of at locations off the construction site and outside the limits of view from the traveled roadway. Sod may be disposed of within the right-of-way, but outside the construction limits, if permitted. Written permission shall be obtained from the property owner on whose property the materials and debris are to be placed. All necessary arrangements shall be made with the owner for obtaining suitable disposal locations. The cost involved shall be included in the contract price of pay items.

50 All merchantable timber in the clearing area, which has not been removed from the right-of-way prior to the beginning of construction, shall become the property of the Contractor, unless otherwise provided. The value of the timber shall be taken into account when the bid is prepared.

Low hanging branches and unsound or unsightly branches on trees or shrubs designated to remain shall be removed as directed. Branches of trees extending over the roadbed shall be trimmed to give a clear height of 6.1 m (20 ft) above the roadbed. All trimming shall be done by skilled workers and in accordance with good tree surgery practices.

60 **201.04 Scalping.** Areas where excavations are to be made, or embankments are to be placed, shall be scalped to a maximum of 100 mm (4 in.). Scalping shall include the removal of material such as brush, roots, sod, grass, residue of agricultural crops, sawdust, and decayed vegetable matter from the surface of the ground.

201.05 Hedge Removal. Hedges and shrubs shall be pulled or grubbed in such a manner as to ensure complete and permanent removal.

201.06 Method of Measurement. When specified as a pay item, measurement of this work will be made by one or more of the following methods.

70 **(a) Area Basis.** The work to be measured will be the number of hectares (acres) and fractions thereof acceptably cleared and grubbed within the limits shown on the plans or staked for clearing and grubbing. Areas not shown on the plans or not staked for clearing and grubbing will not be measured for payment.

(b) Lump Sum Basis. If clearing and grubbing is specified as a lump sum pay item, no measurement of area will be made.

(c) Linear Basis. If a linear unit pay quantity is shown in the Schedule of Pay Items, the length will be measured along the construction centerline in stations.

80 **(d) Individual Unit Basis.**

1. The diameter of trees will be measured at a height of 610 mm (24 in.) above the ground. Trees of less than 100 mm (4 in.) in diameter will be classed as brush.
2. Stumps will be measured by determining the average diameter at the cutoff location.
3. Hedge rows will be measured by the station.
4. Scalping will be measured by the hectare (acre).

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5. If the Schedule of Pay Items shows measurement to be on an individual unit basis, the units will be designated and measured in accordance with the schedule of sizes as follows:

	Measured Diameter at Height of 610 mm (24 In.)	Pay Diameter
100	100 to 200 mm (4 to 8 in.)	150 mm (6 in.)
	Over 200 to 300 mm (Over 8 in. to 12 in.)	250 mm (10 in.)
	Over 300 to 610 mm (Over 12 in. to 24 in.)	460 mm (18 in.)
	Over 610 to 910 mm (Over 24 in. to 36 in.)	760 mm (30 in.)
	Over 910 to 1520 mm (Over 36 in. to 60 in.)	1220 mm (48 in.)
	Over 1520 mm (Over 60 in.)	1520 mm (60 in.)

201.07 Basis of Payment. The accepted quantities of clearing and grubbing will be paid for as shown below.

- 110 **(a) Area Basis.** The determined quantities will be paid for at the contract unit price per hectare (acre) respectively for each of the pay items shown in the Schedule of Pay Items.

- 120 **(b) Lump Sum Basis.** If the Schedule of Pay Items show a lump sum pay item, the lump sum price will be paid for all work shown within the construction limits. All clearing the Contractor is directed to perform outside the construction limits, including clearing for utility relocation which is for the benefit of the Department, and not simply for the Contractor's convenience, will be paid for in accordance with 104.03 or 109.03 unless such clearing is shown on the plans or in the Contract Information book.

(c) Linear Basis. If linear quantities are shown in the Schedule of Pay Items, the quantities will be paid for at the contract unit price for such pay item.

(d) Individual Unit Basis. If individual unit quantities are shown in the Schedule of Pay Items, the accepted quantities will be paid for at the contract unit prices for the respective pay items.

- 130 Payment for tree removal sizes as designated in requirement 5 of 201.06(d), which are larger than those sizes shown as pay items, will be made on the basis of the largest size shown in the Schedule of Pay Items except as set out below.

Payment will be made under:

Pay Item Metric	Pay Unit Symbol (English Pay Unit Symbol)
Clearing and Grubbing	ha (ACRE)
	STA
	LS

30 approved manner. Those which cannot be backfilled satisfactorily shall be removed. If the backfill is within the limits of construction, it shall be completed in accordance with 203.23, unless otherwise directed. All abandoned wells shall be backfilled in accordance with the Indiana Code. A copy of the driller's license shall be furnished prior to commencement of work.

40 Unless otherwise specified, salvageable items removed from the construction site shall become the property of the Contractor and proper allowance for their value shall be taken into account in the bid price of the item involved. Where a house or building has been removed previously and the existing utilities and drains or sewer connections have not been terminated and sealed, this work shall be performed in accordance with 104.03, or as otherwise provided in the contract.

Unless inspection has previously been conducted by the Department, and the findings are shown in the Proposal book, all facilities to be demolished shall be inspected for the presence of regulated materials as defined in 104.06. Facilities are defined as all institutional, commercial, residential or industrial structures, installations, buildings, and all bridges. Inspection and testing for asbestos shall be in accordance with 202.06.1. If inspected by the Department, a copy of the findings will be included in the Contract Information book.

50 At the direction of the Engineer and in accordance with 104.06(b), appropriate tests shall be made by the Contractor of all potentially regulated materials found. The Contractor shall comply with all applicable environmental regulations.

All identified regulated materials shall be reported and removed in accordance with the procedures specified in 104.06 prior to commencing the demolition of the facility. Asbestos removal shall be in accordance with the OSHA Asbestos Standard for Construction Industry, the EPA Asbestos Facts: Demolition and Renovation Regulations and 202.06.1.

60 Except for tank content waste which is classified in 202.07, the Engineer will classify regulated materials, as one of the following Department categories for the purpose of disposal requirements and payment.

(a) Type X Waste. Special waste as classified by IDEM.

(b) Type Y Waste. All waste material that may be disposed of in a Resource Conservation Recovery Act approved landfill.

70 **(c) Type Z Waste.** All waste material that is prohibited from being disposed of in a Resource Conservation Recovery Act approved landfill.

202.03 Removal of Bridges, Culverts, and Other Drainage Structures. Bridges, culverts, and other drainage structures in use by traffic shall not be removed in whole or in part until satisfactory arrangements have been made to accommodate traffic. Any excavation adjacent to the structure or to its approaches shall be shored adequately to avoid damage to them or to traffic.

80 When a reinforced concrete arch bridge is to be removed, either in whole or in part, the work shall include the removal of miscellaneous items within the limits of the structure which must be removed prior to or in conjunction with the removal of the structure. These miscellaneous items shall include but shall not be limited to: concrete and asphalt pavements; concrete and asphalt sidewalks; and fill within the arches regardless of content.

90 **(a) Complete.** Unless otherwise directed, the substructures of existing structures shall be removed down to the natural stream bottom and those parts outside of the stream shall be removed down 0.3 m (1 ft) below natural ground surface. Where such portions of existing structures lie wholly or in part within the limits of a new structure, they shall be removed as necessary to accommodate the construction of the proposed structure. Portions of pre-existing structures that are not visible and not shown on the plans shall be removed as directed. Payment for such removal will be paid as class X excavation in accordance with 206.11.

100 Unless otherwise specified, structural steel and salvageable material shall become the property of the Contractor. It shall be removed from the site before completion of the work and proper allowance for its value shall be taken into account in the bid price of the item involved. If the structure is to remain the property of the Department, steel or wood bridges shall be carefully dismantled without unnecessary damage, steel members shall be match marked, and all salvaged material shall be stored in accordance with 202.02.

Blasting or other operations necessary for the removal of an existing structure or obstruction, which may damage new construction, shall be completed prior to placing the new work.

If stated in the special provisions, all concrete which is of suitable size for riprap and not needed for such use on the project shall be stockpiled on the project in an approved manner, for use by the Department.

110 **(b) Portions.** Portions of the existing structure shall be removed as shown on the plans. Reinforcing bars shall be cut off or allowed to extend into the proposed work as required or as otherwise directed. Explosives shall not be used in the removal of concrete. Where new concrete joins existing concrete masonry, the surface shall be cleaned satisfactorily before new concrete is placed. Adequate safeguards shall be provided to prevent materials from falling below the structure when over or adjacent to traffic; when over bodies of water; as needed to protect life or property; and as needed to comply with laws, regulations, or other contract requirements. A plan shall be submitted for approval showing the proposed method of protection.

120 Pneumatic hammers, up to a maximum of 20 kg (45 lb), may be used for all removal areas to be patched in the deck and all areas within 610 mm (24 in.) of full depth removal lines. Pneumatic hammers up to 31 kg (69 lb) maximum weight may be used for removal of all parapet walls having a construction joint separating the wall from the coping and all partial curb removals. Pneumatic hammers up to 41 kg (90 lb) maximum weight may be used for all other removals outside these limits. Concrete splitters may be used for partial concrete removal subject to satisfactory performance. Deck areas that are to be removed full depth shall be completely separated from adjacent concrete by sawing before hammers heavier than 20 kg (45 lb) may be used.

130 Concrete superstructure or deck removal may be accomplished by pneumatic hammers larger than 41 kg (90 lb), except directly over structural members that are to remain in place. Partial concrete removal of columns, piers, and abutments may be accomplished with pneumatic hammers larger than 41 kg (90 lb), provided that the reinforcing steel in the portion to be removed is completely separated from the concrete that is to remain in place. Alternate methods of removal may be permitted if requested in writing.

140 Hydrodemolition may be permitted for removal of portions of bridge structures as an alternate method to pneumatic hammers. Hydrodemolition for such removals may be accomplished either by use of a machine or a hand held device. Hydrodemolition shall otherwise be in accordance with 722.05(a)2.

Any portion of the structure that is removed, but which was not included within the limits of the concrete to be removed as shown on the plans or as directed, shall be replaced with no additional payment. If at any time during the removal process the tools or methods being used appear to cause any damage to concrete that is to remain, the work shall cease immediately and shall not resume until the Engineer is assured the tools or methods used will not cause further damage.

150 **(c) Disposal of Concrete.** All concrete from complete or partial removals, which is determined to be acceptable for riprap, shall be used on the project as directed. Disposal or placement as riprap will not be paid for directly, but the cost thereof shall be included in the cost of removal.

202.04 Removal of Pipe and Tile Drains. When so provided in the proposal, all pipe and tile drains shall be removed and reasonable precaution taken to avoid breaking or damaging them. The pipe or tile shall be stored neatly on the right-of-way, unless it is to be relaid as a part of the contract. Otherwise, the conditions in accordance with 104.05 shall apply.

160 Pipes to be relaid shall be removed and stored so that there is no loss or damage to the pipe. Replacement will be required of sections lost from storage or from damage through negligence or from improper methods in handling. Removal of pipe or drain tile, any necessary cleaning, removal of headwalls, storage of pipe, and disposal of removed headwall material and unsuitable pipe will not be paid for directly, the cost thereof to be included in the various pay items.

Sanitary or storm sewers no longer in use shall be removed from under the roadway and shoulders if so specified on the plans or in the proposal or if so directed. No payment will be made for this removal if the removal is shown on the plans and no pay item exists, or if this removal is necessary during the placing of other structures or during other excavation operations. The removal of pipes that are not shown in the contract documents and those that are not being replaced at the same location will be paid for in accordance with 109.05.

202.05 Removal of Pavement, Sidewalks, Curbs, Surface, Surface Milling, etc.

All concrete pavement, base course, sidewalks, curbs, gutters, etc., designated for removal shall be:

- (a) broken into pieces and used for riprap on the project;
- (b) broken into pieces, the maximum weight of which shall be 68 kg (150 lb), and incorporated into the work as directed; or
- (c) otherwise disposed of in accordance with 202.02.

Removal of pavement shall consist of the removal and satisfactory disposal of portland cement concrete pavement; portland cement concrete resurface with its base; or the total of any combination of HMA base, intermediate, and surface course of any pavement on a portland cement concrete base, including the base. Each complete pavement removed will be considered as a separate item and paid for as such when removed. Pavement removal shall include only the removal and disposal of existing public road, street, and alley pavement as required for the planned construction. Curb removal shall include curb that is separate from the pavement or removed separately. Integral curb that is removed with the adjacent pavement shall be paid for as pavement removal. Prior to performing the work of pavement removal at locations shown on the plans or where directed, cement concrete pavement to be removed shall be cut with a power driven concrete saw along designated lines. Sawing shall be such that any portion of the pavement to remain in place will not be damaged. Any portion that is damaged or removed outside the designated lines shall be replaced with no additional payment. Sawing of pavement to be removed will not be paid for directly, but shall be included in the cost of pavement removal.

Removal of surface shall consist of removal and satisfactory disposal of the asphalt surface course or courses in place, at an average depth specified. This work, unless otherwise specified, consists of removal of asphalt overlay from a portland cement concrete base. Prior to performing removal of surface at locations indicated, or where directed, the surface course shall be cut to the depth of removal with a power driven saw along designated lines. Sawing shall be such that any portion of the pavement to remain in place will not be damaged. Any portion that is damaged or removed outside the designated lines shall be replaced with no additional payment. Sawing of the surface course will not be paid for directly, but shall be included in the

removal pay item. The removal operation shall be performed in a workmanlike manner, and the area shall be broom cleaned prior to reopening to traffic or before continuing construction operations.

Surface milling shall consist of preparing a foundation for resurfacing by removing the existing surface at an average depth specified or as directed for removal of existing wheel rutting. Unless otherwise specified, the crown of the existing pavement shall be maintained. Surface milling shall not extend deeper than the depth of an existing asphalt overlay, except as directed. On public road approaches, the surface shall be milled at the rate of 25 mm depth per 3 m (1 in. depth per 10 ft) to match the mainline elevation after it has been milled. Surface milling shall also consist of cutting a notch in an existing surface for terminating a HMA surface overlay. HMA overlays shall be placed on all milled surfaces as soon as practical. However, unless otherwise approved, no milled surface shall be left open to traffic for longer than 14 calendar days. If a mainline milled surface is not overlaid after 14 calendar days, \$500.00 will be assessed as liquidated damages, not as a penalty, but as damages sustained for each calendar day that the milled area remains left open to traffic.

Surface milling shall be performed with a power operated planing machine or grinder, equipped with automatic control devices to establish profile grades by referencing from either the existing pavement or from independent grade control. Automatic control devices are not required on surface milling equipment used for notches and public road approaches. The equipment shall have a positive means of controlling cross slope elevations, have an effective means for removing excess material from the surface which prevents airborne dust escaping from the operation, and produce a finished surface that provides a good bond to the new overlay. Sufficient cutting teeth shall be on the cutting drum to produce cuttings such that 90% of the conglomerate particles pass a 50 mm (2 in.) sieve. The roadway shall be cleaned with a broom before opening to traffic, and shall have a surface finish that does not vary longitudinally more than 6 mm (1/4 in.) from a 4.9 m (16 ft) straightedge. If the milling operation results in a vertical or near vertical face exceeding 38 mm (1 1/2 in.) in height, the adjacent lane shall be planed during the same day, or tapered in an approved manner. When located within 75 mm (3 in.) of curbs, surface material that cannot be removed by the planing machine shall be removed by hand or other approved methods. Castings located in surface removal areas that will not require vertical realignment or other adjustment shall remain in place during the milling process, or optionally may be removed and replaced after the milling operation. Payment for this or any other additional work performed by the Contractor at its option shall be included in the cost of surface milling. Unless otherwise specified, the removed surface material shall become the property of the Contractor.

202.06 Removal of Houses and Buildings. This item consists of the satisfactory demolition, removal, backfilling, and disposal of all houses and buildings at locations shown on the plans or where directed. The houses and buildings shall be demolished and removed down to a point 0.3 m (1 ft) below the original ground level or the subgrade elevation, whichever is lower. All accumulated debris in existing basements shall be removed and disposed of. Prior to starting demolition operations, or when directed, all existing utilities shall be terminated and all floor drains shall be sealed in a

satisfactory manner. Temporary fence in accordance with 107.14 may be required where specified or directed. Basements or depressions left by demolition shall be backfilled with B borrow and compacted in accordance with 203.23. No additional payment will be made for temporary fence, the cost thereof to be included in the lump sum price for removal at the location. Temporary fence shall remain the property of the Contractor.

270 The removal of houses and buildings shall be arranged and prosecuted such that all Department maintained highways, and all local roads, streets, and alleys within the project limits shall remain open to normal traffic at all times unless otherwise directed.

Demolition and removal of any individual house or building shall not be started without written authorization. Compensation will be paid only for houses and buildings which are actually removed from the right-of-way as authorized. Removed materials shall be disposed of in accordance with 104.05 and 104.06.

280 In the event the houses and buildings listed for removal from a designated parcel are not in existence at the time of submission of the bid, the lump sum bid for that item shall be indicated at zero dollars and cents.

202.06.1 Inspection and Removal of Asbestos. The Contractor shall comply with all applicable environmental regulations including but not limited to those as follows:

290 (a) 326 IAC 14-10 requires notification to the IDEM of all demolition or renovation operations. Notification is required for renovation operations only if reportable quantities of asbestos are broken, dislodged or disturbed during the renovation procedure. Initial notification to IDEM shall be by certified mail, return receipt requested, or by hand delivery. Verification of this notification shall be provided to the Engineer. The Contractor shall provide such notification 10 work days prior to the date on which removal or demolition operations are anticipated to begin. During the 10 work day period, the IDEM may make a determination of the existence of asbestos materials. If the Contractor postpones the beginning date of demolition or removal operations, the IDEM shall be provided written notice of the new start date, postmarked at least five work days or delivered at least two work days before removal or demolition begins. Verification of this notification shall also be provided. Local governmental agencies may have additional regulations that must be followed. The Contractor shall contact IDEM's air management office to determine what local agencies have regulations.

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(b) 326 IAC 18-3 which requires the inspector conducting the required inspection to be certified by the IDEM. An accredited asbestos project supervisor shall be required to be present at all asbestos removal projects in accordance with 326 IAC 14-10 and 18-1.

(c) Federal Asbestos National Emission Standard for Hazardous Air Pollutants.

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(d) Structurally Unsound and in danger of imminent collapse Building Regulations, in accordance with 326 IAC 14-10-1(b).

202.07 Removal of Underground Storage Tanks Containing Petroleum Products or Other Hazardous Chemicals. Removal of underground storage tanks shall consist of the proper excavation; removal of the tank; removal and disposal of liquids, sludges, and other materials in the tanks; backfilling, and permanent closure of underground storage tanks located as shown on the plans or as identified by the Engineer.

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This work shall be performed in accordance with the requirements as follows:

(a) Technical Standards and Corrective Action Requirements for Owners and Operations of Underground Storage Tanks (UST), Code of Federal Regulations, Title 40, Part 280 (40 CFR 280), Subparts F and G;

(b) American Petroleum Institute Recommended Practice 1604, "Removal and Disposal of Used Underground Petroleum Storage Tanks";

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(c) American Petroleum Institute Publication 2015, "Cleaning Petroleum Storage Tanks";

(d) Resource Conservation and Recovery Act and the Indiana Environmental Management Act;

(e) Underground Storage Tank (UST) Notification, Reporting and Closure Requirements as prepared by the IDEM Underground Storage Tank Branch;

(f) safety regulations issued by OSHA;

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(g) Indiana Fire Prevention Code, Flammable and Combustible Liquids, Article 79, 675 IAC 22;

(h) all applicable Federal and State requirements for certification of underground storage tank removal contractors; and

(i) local fire codes.

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An individual who has been certified for underground storage tank closure or removal, as appropriate, through the State Fire Marshall shall be present at all times for tank closure or removal. Evidence of such certification shall be given to the Engineer prior to starting work.

The removal and disposal of all regulated materials in or around the tanks shall be in accordance with 104.06.

The Contractor shall have the responsibilities as follows:

- 360 (a) obtain a review of available tank information from the Engineer;
- (b) Unless the Department has already done so, provide notification of tank removal operations to appropriate authorities. Notification shall be provided as required to the IDEM, Office of the State Fire Marshal and local fire department in accordance with (a) through (i) above. Notification shall be provided to IDEM at least 30 days prior to closure or removal of regulated tanks in the form of the completed Notification for Underground Storage Tanks Form, and at least 14 days prior to removal or closure to the State Fire Marshall and the local fire department. At least 14 days prior notice, by telephone, shall be given to the IDEM Underground Storage Tank Branch of intended closure or removal date. Such forms are available from the Indiana Department of Environmental Management;
- 370 (c) allow the Engineer to visually inspect tanks after removal;
- (d) allow the Engineer to visually inspect the excavation zone for contaminated soils;
- 380 (e) obtain from the Engineer the limits of excavation for each tank to be removed;
- (f) allow the Engineer to verify all documentation for remediation;
- (g) sample and classify the tank contents, if access is available, or confirm tank contents by sampling and testing;
- (h) submit a site operation plan for the contaminated area to the Engineer for review and approval before beginning removal operations;
- 390 (i) provide and maintain pedestrian safety fencing;
- (j) remove all liquids and sludges from tanks;
- (k) clean tanks and connected piping, including feed lines and drain lines, of contents;
- (l) remove tanks from the ground;
- 400 (m) dispose of all tank content wastes in accordance with the directions provided by the Engineer in 104.06;

(n) render tanks useless or dismantle tanks and transport to scrap dealer or landfill;

(o) implement the site operation plan for the contaminated area as directed in accordance with 104.06;

(p) backfill excavations in an approved manner. Backfill shall be B borrow in accordance with 211;

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(q) maintain accurate records of all operations. Submit reports, including a completed Notification for UST and an UST System Closure Site Assessment Report, to IDEM's UST Branch within 30 days after closure. A copy of these forms shall be provided to the Engineer with verification that the documents were submitted to IDEM;

(r) obtain disposal approvals for the hauling and disposal of all tank content waste materials from the site; and

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(s) if the soil or groundwater surrounding the tank shows evidence of contamination, the hole shall be covered to prevent contamination of rainwater until remediation is complete.

The Engineer will classify the tank contents as one of the following liquid wastes for purposes of disposal requirements and payment.

(a) Type A Waste. Type A waste will consist of direct discharge wastewater which may be discharged to a sanitary sewer system with or without treatment, upon receipt of required permits.

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(b) Type B Waste. Type B waste will consist of low flash wastewater which shall be treated off-site at a treatment facility prior to disposal.

(c) Type C Waste. Type C waste will consist of petroleum or other chemical liquid and sludge wastes which are regulated materials under current EPA, U.S. Department of Transportation, or IDEM regulations. Such waste shall be disposed of at a Resource Conservation and Recovery Act approved facility.

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202.08 Remediation of Contaminated Soil and Groundwater. This work shall consist of remediation. All work shall be performed in accordance with all applicable Federal, State, and local requirements, and 104.06.

Prior to commencing work, the Contractor shall provide evidence satisfactory to the Engineer that the firm and personnel which are performing the remediation are properly trained or certified as required. The Contractor shall have the equipment for the proper remediation of regulated materials. The Contractor shall be familiar with the required procedures and practices governing such work.

The Contractor shall have the responsibilities as follows:

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(a) notify the appropriate authorities regarding remedial operations and provide verification to the Engineer;

(b) take samples and conduct tests as approved by the Department to determine extent of the contamination;

(c) develop a remediation plan and obtain approval for the plan from the Department and the proper authorities;

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(d) remediate the site upon plan approval;

(e) verify that remediation has been completed by conducting the appropriate sampling or testing;

(f) backfill excavations and restore ground lines as directed, in accordance with 211;

(g) maintain accurate and complete records of all operations; and

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(h) submit reports to the Engineer and the proper authorities as requested for proper cleanup documentation.

202.09 Remediation of Other Regulated Materials. This work shall consist of the remediation of regulated materials not otherwise described herein. This work shall include all necessary excavation, backfilling of resultant excavations, and other handling or storage required.

All work shall otherwise be performed in accordance with all applicable Federal, State, and local requirements, 104.06, and 202.08.

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202.10 Transportation and Disposal of Regulated Materials. This work shall consist of determining locations for disposal, treatment, or recycling of regulated materials removed from the project site. This work shall also consist of loading regulated materials into a vehicle or transport container and the movement of such material from the project site to a state or EPA permitted disposal site, storage treatment, or recycling facility by appropriately trained and licensed personnel.

The Contractor shall have the responsibilities as follows:

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(a) determine the location for disposal, treatment, or recycling of regulated materials removed from the project site; obtain written approval of the site; arrange with the approved site for the acceptance of the materials; and obtain the Engineer's written approval for the use of the site prior to transporting the materials;

- (b) ensure that all packing containers or tank vehicles are in accordance with the applicable Federal, State, and local requirements;
- 500 (c) prepare a shipping paper or manifest, as required by Federal and State requirements, for signature of the Engineer or designated Contractor representative;
- (d) ensure that the shipping paper or manifest is carried in the vehicle;
- (e) ensure that all required placards are properly displayed on the vehicle;
- (f) ensure prompt movement of the vehicle to the disposal site; and
- 510 (g) return one copy of the signed shipping or manifest documents to the Engineer.

202.11 Blank.

202.12 Method of Measurement. If the contract stipulates that payment will be made for removal of obstructions or of houses and buildings, or for clearing right-of-way on a lump sum basis, the pay items for such removals will include all structures and obstructions encountered within the right-of-way in accordance with the requirements herein. No measurement will be made. If it is specified that payment will be made for the removal of specific obstructions on a unit basis, measurement will be

520 made by the unit specified in the Schedule of Pay Items. Material used to backfill excavated areas as directed will be measured by the cubic meter (cubic yard).

If the contract stipulates that payment will be made for removal, transportation, or disposal of regulated materials on a unit basis, measurement will be made by the unit stipulated in the Schedule of Pay Items. However, removal of regulated asbestos, if found, will be measured by the square meter (square foot).

Underground storage tank removal will be measured per each within the size groupings of under 11,400 L (3,000 gal.), from 11,400 through 22,800 L (3,000 through 6,000 gal.), over 22,800 through 39,000 L (6,000 through 10,000 gal.) or over 39,000 L (10,000 gal.). Testing for regulated materials will be measured per each for the type and number of tests required.

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The length of pipe removed will be measured by the meter (linear foot), computed by multiplying the number of commercial lengths removed by the nominal laying length, or by measuring in place prior to removal, if practicable.

Removal of present structure or portions thereof will not be measured for payment.

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Surface removal will be measured by the square meter (square yard) of the area removed.

Surface milling, asphalt; and surface milling, portland cement concrete will be measured by the square meter (square yard) of the milled area.

550 **202.13 Basis of Payment.** The accepted quantities of removal of structures and obstructions within the construction limits will be paid for at a contract lump sum price. All structures or obstructions the Contractor is directed to remove outside the construction limits, including clearing for utility relocation which is for the benefit of the Department, and not simply for the Contractor's convenience, will be paid for in accordance with 104.03 or 109.03 unless such clearing is shown on the plans or in the Contract Information book. Such price shall be full compensation for removing and disposing of obstructions in accordance with requirements herein. Regulated materials shall be subject to 104.06. If no contract price is listed in the Schedule of Pay Items for a pay item set out in this specification, no direct payment will be made for work necessary to comply with the requirements for such pay item, except as set out herein. The cost thereof shall be included in the costs of other pay items. If unknown regulated materials are discovered during the contract time, payment for all work relating to
560 removal, testing, transportation, or disposal of such materials will be in accordance with 104.03.

Specific obstructions, including pipe stipulated for removal and disposal, which are shown as pay items, will be paid for at the contract unit price per the unit specified in the Schedule of Pay Items.

Removal of houses and buildings will be paid for at the contract lump sum price for houses and buildings, of the parcel number shown in the Schedule of Pay Items, remove.
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Testing for regulated materials will be paid for at the contract unit price per each for the type and number of tests required. Testing shall include collecting of samples and all necessary laboratory procedures.

Payment for removal of contaminated soils will be based on the actual cubic meters (cubic yards) removed, or by the number of 210 L (55 gal.) drums filled with the contaminated soil.

580 B borrow required for backfilling basements or depressions left by demolition will not be paid for separately but will be included in the cost of the removal item. B borrow required for backfilling of removed contaminated soils or tanks will be paid for in accordance with 211.10.

Underground storage tank removal will be paid for at the contract unit price per each tank within the size groupings of under 11,400 L (3,000 gal.), from 11,400 through 22,800 L (3,000 through 6,000 gal.), over 22,800 through 39,000 L (6,000 through 10,000 gal.), or over 39,000 L (10,000 gal.). Underground storage tank liquid waste disposal will be paid for based on the type of waste and the actual number of
590 liters (gal.) of liquid and sludge removed.

Transportation disposal, and removal of regulated materials will be paid for based on the type of regulated material and the pay unit shown in the Schedule of Pay Items. If such pay unit is specified as drum, the term drum will mean the contents of a 210 L (55 gal.) drum.

600 Clearing right-of-way within the construction limits will be paid for at a contract lump sum price. All clearing the Contractor is directed to perform outside the construction limits, including clearing for utility relocation which is for the benefit of the Department, and not simply for the Contractor's convenience, will be paid for in accordance with 104.03 or 109.03 unless such clearing is shown on the plans or in the Contract Information book.

Removal of present structure will be paid for at the contract lump sum price for present structure, for the structure number specified, remove. Removal of present structure portions will be paid for at the contract lump sum price for present structure, for the structure number specified, remove portions.

610 When directed, portions of the present structure contiguous to the areas shown on the plans or non-contiguous portions of the same character as the planned removal shall be removed. Such additional portland cement concrete acceptably removed will be paid for as measured in its original position, at the contract unit price per cubic meter (cubic yard) for class A concrete in superstructures, class A concrete in substructures, class C concrete in superstructures, or \$427.00 per cubic meter (\$326.00 per cubic yard), whichever is lowest.

Surface milling will be paid for at the contract unit price per square meter (square yard) for surface milling, asphalt; or surface milling, portland cement concrete.

620 Removal of surface will be paid for at the contract unit price per square meter (square yard) for surface, remove.

If there is no pay item for pavement removal and such is encountered, payment will be made for each square meter (square yard) removed. Such pavement removal shall apply only to portland cement concrete pavement or base. A unit price for this work will be established based on thickness, quantity, and removal process. Such unit price will be generated prior to the work being performed. If portland cement concrete pavement has an asphalt overlay, its removal will be considered as incidental, for which no direct payment will be made.

630 Payment will be made under:

Pay Item	Metric Pay Unit Symbol (English Pay Unit Symbol)
Clearing Right-of-Way	LS
Contaminated Soil, Remove	m3 (CFT)
Houses and Buildings, Parcel No. _____, Remove	LS

Underground Storage Tank, Remove and Dispose,
Over 39,000 Liters Capacity EACH
(Underground Storage Tank, Remove and Dispose,
Over 10,000 Gallons Capacity..... EACH)

690 The costs of removal and disposal of buildings, foundations, debris and unsuitable material, guide posts, delineator posts, temporary road material, existing asphalt patches, the filling of abandoned wells; terminating utilities; sealing floor drains where necessary; breaking basement floors; furnishing and erecting all barricades, fences, and other safety measures necessary for adequate protection of the sites; and backfill of basements or depressions left by demolition shall be included in the costs of the pay items of this section. All fence posts and concrete footings shall be completely removed and the resulting holes backfilled accordingly.

If no contract price is listed in the Schedule of Pay Items for work set out herein, no direct payment will be made for compliance with the requirements for such work, except as set out herein. The costs thereof shall be included in the costs of other pay items.

700 If the houses and buildings listed for removal from a designated parcel are not in existence at the time of the letting, no payment will be made for removal work on such parcel.

710 The costs of removing the tanks and all pipe from the ground, removal and disposal of all miscellaneous parts associated with the tank such as concrete pads or holding devices, filing of all required notifications, preparation and implementation of a site operation plan, excavation of all materials necessary in order to remove the tank, compliance with closure requirements, all necessary pedestrian safety fencing, cleaning and draining of tanks and pipes, dismantling or transport, and all required record keeping or reports shall be included in the cost of underground storage tanks, remove and dispose. However, disposal of waste contents and removal of contaminated soil will be paid for separately. No payment will be made for work not performed in accordance with the specifications or not required by the contract.

The costs of all on-site or off-site storage of the materials shall be included in the costs of transportation.

720 All disposal fees and recycling or treatment costs required for regulated materials found within the project limits shall be included in the cost of regulated materials, dispose. If regulated materials are treated on site and not disposed of at an approved location, payment will be in accordance with 104.03.

The costs of removal of all regulated asbestos-containing materials and all safety procedures shall be included in the costs of regulated materials, remove.

The costs of packaging regulated materials, excavation, restoring ground lines, and maintaining and filing required documents and reports shall be included in the costs of the pay items.

730 The cost of removal of regulated asbestos containing materials shall include only the removal of material identified in the contract or by the Engineer as regulated asbestos-containing material. Regulated asbestos-containing materials include the following:

- (a) friable asbestos-containing material;
- (b) Category I nonfriable asbestos-containing material that has become friable or will be subjected to sanding, grinding, cutting, abrading, or burning;
- 740 (c) transite-like material; and
- (d) other Category II nonfriable asbestos-containing material that has a high probability of becoming or has become crumbled, pulverized, or reduced to powder by the forces expected to act on the material in the course of normal demolition operations.

750 Unless directed by the contract or the Engineer, the cost of asbestos removal shall not include the removal of Category I or II nonfriable asbestos-containing material that is not friable or does not have high probability of becoming friable but which becomes friable because the Contractor uses demolition methods that cause such materials to become regulated. Such costs shall be included in the cost of other pay items.

760 The costs of all labor; equipment; materials; and documentation required for complying with the applicable laws, regulations and procedures, including but not limited to, licenses, permits, other legal fees, or disposal charges shall be included in the costs of the pay items. No payment will be made for work which is not performed in accordance with the specifications, nor that which is not required by the contract.

760 The cost of removal of specific work shall include the removal and disposal of such obstructions, the necessary excavation required, salvage of materials removed, their custody, preservation, storage on the right-of-way, and disposal as provided herein. All damage to existing facilities caused by the Contractor's operations or equipment shall be satisfactorily replaced or repaired with no additional payment.

770 The cost of clearing right-of-way shall include the cost of all work described herein except for that which is set out specifically as pay items, or work which is described in 104.06, 202.07, 202.08, 202.09, 202.10, or 202.11.

If it is necessary to package the contaminated soil in a container, the cost of the container and all costs related to packaging shall be included in the cost of removal. The costs of all excavation pertaining to contaminated soil, removal of all soil within the limits established by the Engineer, restoring ground lines, maintaining required records and filing of reports shall be included in the cost of contaminated soil, remove. No payment will be made for work beyond the limits established by the Engineer, work not performed in accordance with the specifications, or work not required by the contract unless in accordance with 104.03.

780 The costs of all handling of the product, removal of the product from the tank, disposal, all required packaging, and transportation shall be included in the cost of underground storage tank, liquid waste disposal.

The costs of milling material and the removal of such material from the project site shall be included in the cost of surface milling.

All necessary cleanup of spills caused by the Contractor will not be paid for.

SECTION 203 -- EXCAVATION AND EMBANKMENT

203.01 Description. This work shall consist of embankment construction and excavation, hauling, and disposal or compaction of all material not being removed under some other item which is encountered within the limits of the work and also from intersecting entrance approaches beyond the right-of-way limits necessary for the construction of the road way in accordance with the specifications and in reasonably close conformance with the lines, grades, thickness, and typical cross sections shown on the plans or as directed. All excavation will be classified as hereinafter described.

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203.02 Common Excavation. Common excavation shall consist of all excavation not included as rock excavation or excavation which is otherwise classified and paid for, including flexible type pavement and all rippable materials.

203.03 Rock Excavation. Rock excavation shall consist of igneous, metamorphic, and sedimentary rock which cannot be excavated without blasting; the use of a power shovel of not less than 0.76 m³ (1 cu yd) capacity, properly used, having adequate power and in satisfactory running condition; or the use of other equivalent powered equipment. Rock excavation shall also include all boulders or other detached stones each having a volume of 0.4 m³ (1/2 cu yd) or more.

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203.04 Unclassified Excavation. Unclassified excavation shall consist of the excavation and disposal of all materials of whatever character encountered in the work.

203.05 Peat Excavation. Peat excavation shall consist of the necessary excavation and satisfactory disposal of peat, muck, marl, or any other similar unsuitable material in peat deposits, together with any overlaying material, except pavement, which is not used in embankment construction, except as otherwise provided in 203.16.

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203.06 Waterway Excavation. Waterway excavation shall consist of the necessary excavation and satisfactory disposal of all material resulting from excavation for clearing waterways, making channel changes, or both when such are itemized in the Proposal Book, but shall not include class Y excavation, or excavation made for a structure in accordance with 206. If not otherwise specified, waterways shall be cleared for the entire distance within right-of-way lines.

40 **203.07 Class Y Excavation.** If there be encountered within the limits of waterway excavation material which can be classified as rock in accordance with 203.03, or material which consists of conglomerate, concrete, masonry, or any similar material which is not part of an existing structure shown on the plans, shall be defined as class Y excavation. Material as defined in 203.02 will not be considered as class Y excavation.

50 **203.08 Borrow or Disposal.** Borrow shall consist of approved material required for the construction of embankments or for other portions of the work and shall be obtained from approved locations and sources outside the right-of-way. Borrow material shall be free of substances that will form putrescent or deleterious deposits, or produce toxic concentrations or combinations that may be harmful to human, animal, plant or aquatic life, or otherwise impair the designated uses of the stream or area. Unless otherwise designated in the contract, arrangements shall be made for obtaining borrow. Borrow, as designated herein, shall not include material excavated beyond the right-of-way limits at intersecting public roads, private and commercial drive approaches, nor material furnished as B borrow.

Before borrow or disposal operations are begun, a borrow pit plan or a disposal operations plan shall be submitted for approval. Such plan shall include the following:

- 60 (a) a detailed sketch showing the limits of excavation for the borrow pit relative to property and right-of-way lines;
- (b) all slopes shall be designated;
- (c) an erosion control plan which includes measures to keep sediment from entering streams. It shall show diversion channels, dikes, sediment traps, or other measures which may be used for this purpose; and
- 70 (d) the fine grading, encasement, and seeding procedures which shall be performed on the pit when work is completed.

Notice shall be given in advance of opening borrow areas so that cross section elevations and measurements of the ground surface after stripping may be taken and the borrow material may be tested before being used.

The borrow pit limits, except when borrow is to be obtained from a commercial source, shall be identified and have a qualified archaeologist perform a record check and an intensive field survey to determine if any significant archaeological sites are within the borrow pit limits. Results of the record check and survey shall be furnished in writing prior to the excavation of any material. If any archaeological sites are identified, the archaeologist will establish the limits of the site along with a reasonable border. The site shall not be disturbed unless the archaeological site is cleared by established procedures and written authorization to enter the site has been issued. Under no circumstances shall an employee of the Contractor or the State of Indiana share in the ownership or profit from the sale of any archaeological artifacts that may be salvaged. No extension of completion time will be granted due to any delays in securing approval of a borrow pit.

Unless written permission is granted, there shall be no excavation in a borrow pit below the elevation of the adjacent properties within 45 m (150 ft) of the nearest right-of-way line of an existing highway, county road, or city street; the nearest right-of-way line of a proposed highway, county road, or city street; or adjacent property lines. If the properties adjacent to the borrow pit are privately owned, the setback limit of 45 m (150 ft) may be lessened if written approval or permission is granted by the owner of the adjacent property, the excavation is in accordance with local zoning laws and requirements, and if lessening the limit is in the best interest of the State. Such minimum distance shall not be closer than 15 m (50 ft) to an adjacent property line. All excavated slopes of a pit shall not be steeper than 3:1 down to 0.6 m (2 ft) below the ground water elevation. All excavated slopes 0.6 m (2 ft) below the ground water elevation shall not be steeper than 2:1.

Good top soil from the borrow pit area shall be saved for use in restoring the excavated area, or similar material from another source may be substituted in order to place a 150 mm (6 in.) minimum encasement on the 3:1 or flatter slopes. Final restoration of borrow or waste disposal areas shall include grading, seeding, and/or other necessary treatments that will blend the area into the surrounding landscape. Restored areas within 45 m (150 ft) of the nearest right-of-way line shall be well drained. Areas beyond 45 m (150 ft) shall be drained unless the landowner desires other treatment of the borrow pit area. No payment will be made for restoring areas as set out herein, the cost thereof to be included in the contract unit price for borrow. Drainage, location, or use of the pit shall be in accordance with existing laws, regulations, and ordinances. Under no conditions shall borrow sites detract from the appearance of the natural topographical features nor increase the potential hazard to a vehicle that has inadvertently left the highway.

If granulated slag, dunes sand, or other granular material which is not suitable for the growth of vegetation is used, such material shall not be placed within 0.3 m (1 ft) of the required finished surfaces of shoulders and fill slopes. Additional material required to complete the embankment, such as sandy loam, sandy clay loam, clay loam, clay, or other materials suitable for the growth of vegetation and free from clods, debris, and stones, shall be furnished at the contract price for borrow.

With written permission and as directed, additional fill material may be secured from within the permanent or temporary right-of-way in lieu of borrow or B borrow either from vertical or horizontal extensions, or both, beyond the lines and elevations of roadway and drainage excavation as shown on the contract plans. This work shall not be performed until written approval has been obtained and the conditions and basis of payment in accordance with the specifications and proposal have been accepted. If the additional material has been obtained without this written approval, such material will be classified, either as to source or use, to the best advantage of the Department.

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CONSTRUCTION REQUIREMENTS

203.09 General Requirements. The excavation and embankments for the roadway, intersections, and entrances shall be finished to reasonably smooth and uniform surfaces. No materials shall be wasted without permission. Excavation operations shall be conducted so that material outside of the limits of slopes will not be disturbed. Prior to beginning excavation, grading, and embankment operations in any area, all necessary clearing and grubbing in that area shall have been performed in accordance with 201.

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All spongy and yielding material which does not readily compact, and all vegetation shall be removed from within slope-stake limits and to such depths as ordered. Soft or unstable materials which are encountered where the proposed embankment will be placed, shall be removed. If ground water is encountered, backfilling shall be accomplished using B borrow in accordance with 211.02(b) to an elevation at least 0.6 m (2 ft) above the ground water level. Compaction of the B borrow placed above the free water level shall be accomplished using heavy vibratory equipment. If groundwater is not encountered during the removal operation, the backfill shall be placed in accordance with the following paragraph. None of the removed materials shall be used in embankment, except if approved, they may be used if aerated such that proper compaction can be achieved.

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After the embankment area has been cleared and before embankment is placed, all pronounced depressions left in the original ground surface by removal of objectionable material from within embankment limits shall be refilled with suitable material well compacted to the same density as required for all other embankment. The upper 150 mm (6 in.) of the original ground shall be compacted with a roller weighing no less than 9.1 Mg (10 t), or with other approved compacting equipment. Proofrolling of the natural ground surface shall be performed in accordance with 203.26 within all areas where new fill will be placed.

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If the original ground cannot be compacted to the required density because of unstable soils, if a high water table condition results in a yielding foundation, or if other similar conditions exist, the use of a stabilizing mat made of B borrow in accordance with 211.02(b) to cross the area is acceptable. The mat shall be a minimum of 0.3 m (1 ft) thick, a maximum of 0.6 m (2 ft) thick, and shall be extended so as to daylight at the toe of slope. Where B borrow is exposed, it shall be capped with 150 mm (6 in.) of No. 2 aggregate to prevent erosion.

170 When it is anticipated that the use of a 0.6 m (2 ft) mat does not satisfactorily stabilize an area, or when preliminary exploration indicates removal of more than 1.2 m (4 ft) of unsuitable material will result in excavating 191 m³ (250 cu yd) or more, or when it is anticipated that the use of a geotextile in conjunction with a B borrow mat will be needed to stabilize an area, the proposed treatment will require approval prior to use.

Payment for excavation of these unsuitable materials shall be in accordance with 203.28, and payment for B borrow shall be in accordance with 211.09 and 211.10.

180 Frozen materials, stumps, roots, all or parts of trees, brush, weeds, sod, or other perishable materials shall not be placed in embankment. Stones greater than 150 mm (6 in.) in any dimension shall not be left within 200 mm (8 in.) of the finished subgrade. The original ground surface, or the surface of any lift in place shall not be frozen and shall be free from objectionable quantities of snow, ice, or mud.

The roadway shall be kept well drained at all times by keeping the center higher than the sides and uniformly graded. If necessary, temporary drainage ditches shall be provided as directed.

190 Each embankment lift shall extend transversely over the entire area and shall be kept smooth. If a dragline or similar equipment deposits material in large unit masses onto embankment, the material so cast shall be moved from its place of deposit and spread out in layers as specified herein for uniform lifts.

The use of hydraulic methods to construct embankments will be permitted only when authorized in writing. Only B borrow shall be placed below the free water level. Backfill at structures shall be compacted in accordance with 211.04.

200 If the same or similar material is being used in the upper lifts of embankment as shall be used in subbase at that location, these lifts shall be placed in smooth uniform layers for the full width of the embankment.

When grading operations are performed after daylight hours, sufficient artificial lighting shall be provided and maintained, with no additional payment, to permit proper construction and inspection.

Grading operations shall be kept well in advance of paving operations in order to take advantage of maximum time for settlement.

210 All slopes which are graded and not immediately stabilized with other erosion control measures shall be roughened as described herein, until permanent erosion control measures are placed. Roughening shall take place each day after work is performed on the slopes, or as directed to reestablish the roughening.

The soil shall be roughened on slopes using implements which may safely operate on the slope to create a series of ridges and depressions which run across the slope on the contour, and make grooves at least 25 mm (1 in.) deep and not more than 405 mm (15 in.) apart.

220 Except as herein provided, sufficient quantities of available excavation suitable for the growth of vegetation shall be preserved from within the planned excavation area and used for the encasement of cut, fill, and shoulder slopes which are not suitable for the growth of vegetation. The depth of encasement shall be 150 mm (6 in.) or more, as directed, measured perpendicular to the face of the slope. No additional compensation will be allowed for this work except payment will be made for the class of excavation involved for authorized undercutting of back slopes. Encasement of rock embankment and cut slopes will not be required unless otherwise directed.

230 Material suitable for the growth of vegetation shall be as approved. It may consist of sandy loam, sandy clay loam, clay loam, clay, or other suitable material free from clods, debris, and stones. The material placed on backslopes of cut sections may be placed in accordance with 203.25.

If sufficient excavation suitable for the encasement of cut, fill, and shoulder slopes is not available, suitable borrow material shall be furnished at the contract unit price for borrow. The source of the borrow material for encasement shall be approved. Payment will be made in accordance with 203.28. If the contract does not contain a pay item for borrow, a change order will be executed for payment of borrow. Suitable portions of common excavation may be preserved or borrow material may be furnished
240 for encasement provided all suitable excavation is used constructively.

203.10 Disposal of Excavated Material (Except Waterway and Peat Excavation). Excavation material shall be used for the construction of embankments, shoulders, special fill, or other places as may be specified or directed, depending on the nature of the material. Excavated material that is suitable for embankment construction shall be placed in the embankment before placing any borrow material, except for the quantity required for the maintenance of traffic, unless otherwise authorized in writing. No material shall be wasted without authority.

250 If more material is excavated from within required cut slopelines than is needed to construct embankments or special fills, the excess may be used to widen embankments, flatten fill-slopes, or be used otherwise as directed. Any excess excavated material that cannot be used constructively within the project limits shall be disposed of off the right-of-way in accordance with 201.03.

Excavation obtained from the right-of-way and planned to be used in fills may be wasted and replaced with borrow with no additional payment only after written permission is obtained. Permission may be granted after receipt of a written request and after it has been determined that granting the request would not be to the disadvantage
260 of the Department. All required samples of the borrow or the excavation materials involved shall be furnished with no additional payment.

203.11 Disposal of Waterway Excavation. Unless otherwise provided, material resulting from waterway excavation shall be used to fill old channels and, if suitable, in embankment, special fill, and approach embankment, or any combination of these, as specified or directed.

270 A portion of waterway excavation which is unsuitable for the above uses, a portion which is suitable but is in excess of that required for such uses, or if locations for such disposal are not available, then disposal shall be in accordance with 201.03.

203.12 Disposal of Peat. All material removed as peat excavation, removed or displaced by machine operation, or displaced by the advancing backfilling material shall be uniformly spread between the toes of fill slopes and the swamp ditches or beyond, or otherwise disposed of as specified or directed.

280 **203.13 Slides.** Slides encountered during construction shall be removed as directed and their removal will be paid for as the class or classes of excavation encountered.

If the contract involves paving, the omission or delay of paving operations may be required at the location of a slide. If proper treatment of a slide has been obtained prior to completion of the remaining pavement, the gap may be required to be paved, and payment will be at the contract unit price for pavement.

If proper treatment of a slide has not been obtained prior to completion of the remaining pavement, the gap left at the slide location shall become an exception to the contract item for pavement.

290 **203.14 Drainage.** Ditches shall be interpreted to mean open ditches and channel changes parallel to and adjacent to the roadbed. Channel changes excavated under the classification of waterway excavation are not included in this definition.

Lines, grades, and cross sections of ditches shall be as shown on the plans, unless otherwise established to obtain proper drainage.

Ditches and gutters emptying from cuts onto embankment shall be constructed to avoid eroding the embankment.

300 Exploration of underground drainage and sinkholes may be required, and payment will be as hereinafter provided. Should any underground drain be encountered, the location of which is not shown on the plans, notification shall be made at once. Drainage shall be explored as directed and, if deemed necessary, taken care of under applicable provisions of these specifications, or as otherwise directed.

310 If existing surface drains, tile drains, sewers, or other underground drains, or parts thereof, are not to be replaced or are not required by the terms of the contract or directed to be changed, whether such drainage facilities are shown on the plans or not, all such drainage facilities or parts thereof shall be protected, preserved and satisfactorily continued in use without change. If in the prosecution of the work such

existing drainage is changed or interrupted, or through negligence such drainage is interrupted or damaged, satisfactory permanent repairs shall be immediately provided or adequate temporary drainage facilities shall be maintained until permanent repairs are made. If temporary facilities are provided, before the work is accepted, such damaged or interrupted drainage facilities shall be restored to the original condition or to an altered state which is at least equal to their original condition.

320 If slopes or ditches which were graded for a grading contract become eroded or scoured during the paving contract work, the scoured or eroded areas shall be reshaped to the original cross section and reseeded or resodded as shown on the plans, all in accordance with 208 and 621.

When so provided by the plans or special provisions, or when ordered, all tile drains, sewers, or other underground drains encountered in the prosecution of the contract shall be repaired, replaced, extended, reconstructed, connected, or otherwise changed.

330 Unless otherwise provided in the contract, the cost of replacing, restoring, or connecting an underground drain which is substantially in its original location and incidental to roadway and drainage excavation, structures, or other drains will not be paid for directly, but the cost thereof shall be included in the cost of various pay items.

Unless otherwise provided, any necessary drainage change or restoration not shown on the plans and not due to negligence or operations of the Contractor will be paid for at the contract unit price or prices thereof. If there is no such contract unit price or prices, such work shall be done and payment made in accordance with 104.03.

340 **203.15 Excavating Rock.** If material is encountered during excavation which appears to belong in the classification of rock excavation in accordance with 203.03, notification shall be made in writing. Ample time shall be allowed to make such investigation and measurements that are necessary to determine the class and volume of the material in question.

350 Exploratory cores shall be taken from the top of the rock to approximately subgrade elevation as directed. The cores shall be cut with standard diamond core bits and series X double tube core barrels to obtain 54 mm (2 1/8 in.) diameter samples. All cores shall be suitably marked and identified to show the location of the core by station, offset from centerline, elevation of top of rock, depth below top of rock, and percent recovery within each core. All cores shall be retained. The cores shall be placed in suitable compartmented wooden boxes in the order in which removed from the boring, with dividers between core runs. The top and bottom of each run shall be appropriately marked. The cores shall be transported to a location as directed.

The top of rock elevations shall be determined prior to locating the top of soil cut slopes where finished rock slopes are planned to be one to one or steeper.

Final breakage of rock excavation shall be in accordance with or closely approximate the slope-lines shown on the plans unless different slope-lines are fixed, and the Contractor so notified. The final slopes shall be left reasonably smooth and uniform, and all loose and overhanging rock removed. Unless otherwise permitted, no rock shall finally project more than 0.3 m (1 ft) beyond established slopes. If natural seams intersect an established slope, the excavation may be carried, with permission, along the face of such seams for the distance approved.

Rock shall be excavated to the required elevation for the full width of the roadbed as shown on the plans or as directed. Where rock is excavated below the required elevation, the area shall be backfilled to the subgrade elevation with crushed stone, spalls, subbase material, or other approved granular material, which shall be shaped and compacted to the required elevation and cross section.

Exploratory drilling, which shall consist of drilling holes for the purpose of determining the existence of cavities affecting underground drainage and possible sinkholes in cut sections, may be required at locations as directed. Unless otherwise directed, the holes shall be drilled on the centerline of the proposed pavement at approximately 30 m (100 ft) intervals and shall extend for a depth of 2.1 m (7 ft) below the proposed grade and have a minimum diameter of 38 mm (1 1/2 in.).

Where cavities are discovered on centerline, additional holes shall be drilled at 8 m (25 ft) intervals along the edge of the proposed pavement, and unless otherwise directed, extend for a depth of 2 m (7 ft) below proposed grade. Where any cavity is discovered or exposed having a minimum cover of less than 1.5 m (5 ft), the cover shall be removed, and the treatment of such areas shall be in accordance with details in the plans or as otherwise directed.

The final surface of rock excavation under the roadbed area shall be left so that drainage between the rock surfaces and any material placed thereon will be substantially complete. Where seams of clay or other soft material 0.3 m (1 ft) or less in thickness are encountered in rock excavation, the volume of such seams will be considered as rock excavation and paid for as such.

Unless otherwise specified or directed, rock shall be pre-split by drilling and the use of explosives in such a manner as results in minimum breakage outside neat lines of the typical cross section and slope stakes as established. Deposits of commercial building stone outside the right-of-way shall not be damaged. Holes for pre-splitting shall be drilled along the established slope-stake lines. The holes shall be from 50 to 100 mm (2 to 4 in.) in diameter and, unless otherwise directed, be spaced approximately 0.9 m (3 ft) apart. Holes shall be drilled approximately 0.6 m (2 ft) below the established grade of the cut, or the predetermined bench elevation, or as directed.

The maximum depth of any pre-split lift shall be 9 m (30 ft), unless otherwise directed or permitted. If more than one lift is required, the first line of drill holes shall be set in such a manner as to allow for a specified offset for each succeeding lift and an offset of 0.6 m (2 ft) from the back of the paved side ditch line. The explosives used and the method of loading depends on the material to be blasted. These explosives may vary from a single strand of detonating cord, for blasting unconsolidated formations, to a solid column of dynamite for massive formations. However, the explosive shall be of a type to accomplish the pre-splitting with a minimum of breakage outside the excavated area. After the charges are placed, the holes shall be filled with sand or other suitable granular material.

Except as indicated below, all pre-splitting charges shall be detonated simultaneously by the use of instantaneous electric blasting caps or by means of a detonating cord trunkline. The line holes shall be fired before the main excavation is blasted. Pre-splitting shall be kept well in advance of regular blasting operations. Primary blasting holes shall be drilled no less than 1.8 m (6 ft) from the pre-split face, unless otherwise permitted or directed. If additional charges are required, holes shall be placed at half the distance of a full depth hole to a depth such that the bottom of the hole clears the pre-split face approximately 0.6 m (2 ft). The pre-split face shall deviate no more than 150 mm (6 in.) from the front line of drill holes or more than 300 mm (12 in.) from the back line of drill holes except where the character of the rock will unavoidably result in some irregularities.

The amount of explosives per shot for instantaneous firing or the amount of explosives per delay for delay firing shall not be great enough to damage nearby structures, rock formations, or other property. Where commercial building stone formations are located in the effective vicinity, adequate seismograph readings shall be obtained, with no additional payment, as evidence that blasting operations have not altered existing commercial building stone formations outside the right-of-way limits of the project.

Permission may be granted to use machine methods to establish the finished slopes in those cuts where machine methods are used to remove roadway excavation, provided final machine finished slopes are equal or superior to that which would be obtained by pre-splitting methods.

When or where any direct or indirect damage or injury is done to public or private property by or on account of any act, omission, neglect, or misconduct in the execution of the work or in consequence of the non-execution thereof, such property shall be restored, with no additional payment, to a condition similar or equal to that existing before such damage or injury was done by repairing, rebuilding, or otherwise restoring as directed, or such damage or injury shall be made good in an acceptable manner.

No direct payment will be made for pre-splitting, but the cost thereof shall be included in the pay item of rock excavation or unclassified excavation.

450 **203.16 Peat Deposit Treatment.** If pavement is specified at a location where a peat deposit is to be treated, the deposit shall be treated as far ahead of paving operations as practicable to take advantage of maximum settlement. If approved settlement has not been obtained when paving is complete up to the limits of any peat deposit treatment, a gap in the pavement shall be left as directed. Such a gap shall be paved later if approved settlement is obtained prior to or at the time paving operations are complete on the remainder of the contract but, if not, the pavement originally planned for the location of such a gap shall become an exception to the contract. If any such gap is subsequently paved, payment will be made at the contract unit price for pavement.

460 Necessary blasting shall be with 40%, 50%, or 60% straight nitroglycerine or gelatine dynamite as directed.

Treatment shall be by one of the following methods or any combination thereof as shown on the plans, or as directed, or both.

(a) Treatment of Existing Fills. If the required alignment is on an existing fill over a peat deposit, treatment may be required by any one or any combination of the following methods:

- 470 1. removal of the existing fill with or without the removal of the underlying peat and of the material at the sides of the existing fill;
2. blasting the peat under the existing fill;
3. loading the existing fill with additional fill material, and, if directed, blasting the peat underneath;
4. leaving the existing fill in place and treating the material at the sides either by removing the peat at the sides and backfilling or by displacement or both.

480 Determination of the method or methods to be used will be made based on conclusions from test holes which may be required to be drilled at designated locations.

(b) Treatment by Removal. This method consists of completely removing the objectionable material either as peat excavation or by machine operation and backfilling to the full toe-width of the proposed embankment or to such other widths, if so directed depending on the condition and depth of the material to be removed.

490 If water is not present, the space previously occupied by the removed material shall be backfilled with common excavation, borrow, or both, and placed in accordance with the applicable provisions of 203.

If water is present, the backfill shall be with material in accordance with 211.02(b). Placement of this material shall follow as closely behind the removal of the peat as possible. It shall be carried across the area from one end to the other by end-dumping and finally left at the established grade. This grade shall be such that keeps end-dumping to a minimum, which nominally shall be approximately 0.6 m (2 ft) above free water level. That portion between free water level and this established grade shall be thoroughly watersoaked to secure maximum compaction.

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If additional fill is needed to bring the embankment to its final required grade, it may be common excavation or borrow. Further placing of the granular material above the end-dumped material may be authorized. This additional fill shall be placed and compacted in accordance with applicable provisions of 203 but shall not be placed for at least 14 days after the end-dumped material is placed and compacted. This period may be shortened or lengthened with written approval, depending on the settlement that has been obtained.

(c) Treatment by Displacement. When this method is used, the peat at each
510 end of the deposit shall be removed completely by excavation to the full width shown on the plans, or to such other width as may be directed, until a point is reached where the depth of the peat being removed is greater than 3 m (10 ft), or to a greater or lesser depth, as directed.

If conditions permit, the upper portions of the remaining peat shall be excavated across the remainder of the deposit in the same manner as set out above for the ends. Removal of this upper portion shall begin at one end and proceed to the other end at a sufficient distance ahead of backfilling operations so displacement of the remaining peat will not be retarded. If excavation to the required depth is not maintained for the full
520 width ahead of the backfill or surcharge, additional units shall be used or backfill operations stopped until the two operations are in balance. If conditions permit, draglines shall be operated from mats in front of the advancing excavation. If blasting is required to aid displacement, it shall be done as extra work in accordance with 104.03.

Backfilling shall be done in accordance with 203.16(b) for end-dumped material except, when required, a temporary surcharge shall be maintained at the head of the backfilling. The top of the surcharge shall be constructed and maintained to a width equal to the full shoulder width of the proposed embankment, or as approved or directed. The height of the surcharge shall be the same as the depth of the peat being
530 treated, unless otherwise directed. The original ground shall be the reference elevation for measuring peat depths and surcharge heights. The top of the surcharge shall be approximately level and the length on top shall be at least two times the depth of the peat being treated. The surcharge shall be kept built up and pushed forward with a bulldozer or other approved equipment as the displacement progresses. Machine methods shall be used to relieve pressure at the advancing toe and sides of the surcharge. Sufficient hauling units shall be used to maintain the surcharge at the required height, width, and length. The machine operation to augment displacement shall be coordinated with the rate of placing temporary surcharge.

540 After the granular backfill has been completed across the deposit, any remaining
fill necessary to bring the embankment up to the required final grade shall be in
accordance with 203.16(b) for that portion above the end-dumped material.

203.17 Cased Test Holes. As displacement progresses, cased test holes shall be
placed at locations as directed to determine the extent of peat displacement and for use
in conjunction with final measurement. These holes shall extend to the bottom of the
deposit. The boring shall be conducted in such a manner that accurate information may
be obtained as to the nature of the materials through which the test holes are placed. If
these test holes indicate that full displacement has not been obtained, the remaining peat
550 shall be blasted or additional treatment shall be performed as required.

Unless otherwise permitted in writing, cased test holes shall be placed by
hydraulic boring. The external casing shall be a minimum of 50 mm (2 in.) in diameter
and of such additional size as to perform the operation satisfactorily. A continuous
supply of fresh water shall be jetted through an internal pipe so that the wash water and
loosened material is carried to the surface between the jet rod and the external casing.
Pumps and other miscellaneous tools and equipment shall be used as required to
perform a satisfactory operation.

560 **203.18 Embankment Construction.** Embankment construction shall consist of
constructing roadway embankments, including preparation of the areas upon which they
are to be placed; the construction of dikes within or outside the right-of-way; the
placing and compacting of approved material within roadway areas where unsuitable
material has been removed; and the placing and compacting of embankment material in
holes, pits, and other depressions within the roadway area. Only approved materials
shall be used in the construction of embankment backfill. Rocks, broken concrete, or
other solid materials shall not be placed in embankment areas where piling is to be
placed or driven.

570 **203.19 Embankment Over and Around Structures.** Fill shall not be placed
against any new masonry abutment, wingwall, retaining wall, or culvert, nor over any
culvert, bridge, or arch until approved. Applicable provisions of 702.23 shall be met.

Filling around culverts, bents, and piers, and fill below the natural ground
surface at abutments, wings, and retaining walls shall be deposited on both sides to
approximately the same elevation at the same time. Piers or bents shall not be displaced
and shall be checked for proper location as the work progresses. Corrective measures
shall be made if necessary. Filling at arch structures shall be carried up in horizontal
layers, symmetrically from haunch to crown, and simultaneously over and against all
580 piers, abutments, and arch rings.

Wedging action of filling material against structures shall be prevented. If
directed, back slopes of excavation shall be destroyed by stepping or serrating.

The embankments around the end bents shall be constructed to approximate subgrade elevation for a distance of no less than 23 m (75 ft). This work shall be done before piling in the end bents are driven, and before the end bent or abutment is constructed. Compaction shall be in accordance with 203.23. After the embankments are completed, the embankments shall be excavated to construct the end bents and berms. Before driving piles, holes to receive the piling shall be cored through the embankment in accordance with 701.04(b)2. After the piles have been driven, the space between the pile and the cored hole shall be backfilled with granular material as directed. If the embankment in the area of the end bents is to consist of sand, gravel, or other permeable material in which a cored hole would not remain open, the piling shall be driven before the embankment is constructed. No direct payment will be made for excavation of the embankment at the end bents or abutments, or for coring, backfilling holes, or excavating fill, the cost to be included in the costs of other pay items.

203.20 Rock and Shale Embankment. Utilization of these materials in embankment construction shall be in accordance with the following.

(a) Rock Embankment. Where rock is used for embankment, no large stones shall be allowed to nest but shall be distributed over the area to avoid pockets. Voids shall be filled carefully with small stones. The final 0.6 m (2 ft) of the embankment just below the subgrade elevation shall be composed of suitable material placed in layers not exceeding 200 mm (8 in.) loose measurement and compacted to the required density. Shale or shale-like materials will not be permitted in the upper 0.6 m (2 ft) of the embankment.

Where the depth of an embankment exceeds 1.5 m (5 ft) and is to consist entirely of rock, the rock shall be deposited in lifts not to exceed the top size of the material being placed, but in no event exceeding 1.2 m (4 ft). The rock for any particular lift shall be deposited on and pushed over the end of the lift being constructed by means of bulldozers or other approved equipment. Depositing of rock over the end of any lift from hauling equipment will not be permitted. If the voids of the last lift are not closed sufficiently, they shall be choked with small broken stone or other suitable material and compacted as directed.

Where the depth of embankment is 1.5 m (5 ft) or less, or where the material being placed does not consist entirely of rock, the material shall be placed in lifts not to exceed the top size of the rock being placed but not exceeding 0.6 m (2 ft). Each layer shall be choked thoroughly with broken stone or other suitable material and be compacted to the required density or as directed.

Where a rock fill is to be placed over a structure, the structure shall first be covered with from 0.6 to 1.2 m (2 to 4 ft) of earth or other approved material, and properly compacted before the rock is placed. This covering shall be placed in accordance with 203.19.

Shale shall not be incorporated as rock embankment unless written permission is obtained.

(b) Shale, Shale and Soft Rock Mixtures, or Soft Rock. When these materials are encountered and are to be used for embankment construction, the compaction shall be accomplished with an approved vibratory tamping-foot roller in conjunction with a static tamping-foot roller. The minimum weight for the static tamping-foot roller shall be 27 Mg (30 t). The minimum total compactive effort for the vibratory tamping-foot roller shall be 244 kN (27.5 t). Total compactive effort is defined as that portion of the static weight acting upon the unsprung compaction drum added to the centrifugal force provided by that drum. If the manufacturer's charts do not list the static weight acting upon the compaction drum, the roller shall be satisfactorily weighed, the weight shall be added to the centrifugal force, and the roller rated in accordance with the Construction Industry Manufacturers Association, CIMA. Each tamping foot on the static roller shall project from the drum a minimum of 150 mm (6 in.). Each tamping-foot on the vibratory tamping-foot roller shall project from the drum a minimum of 100 mm (4 in.). The surface area of the end of each foot on both tamping-foot rollers shall be no less than 3550 mm² (5 1/2 sq in.).

Shale, shale and soft rock mixtures, or soft rock shall be placed in 200 mm (8 in.) maximum loose lifts, and shall be compacted to at least 95% of maximum dry density. The moisture content shall be controlled within -2 and + 1 percentage points of optimum moisture content. The density will be measured with a calibrated nuclear gauge using the direct transmission mode. Excavation and blasting procedures shall accommodate the selective placement of these materials and avoid intermixing rock. Rock shall be placed in accordance with 203.20(a).

Water shall be applied to the shale in the cut to accelerate the slaking action and again prior to disking and compaction to facilitate the compaction. The water shall be distributed by an approved method which provides uniform application of the required quantity of water. The water shall be uniformly incorporated throughout the entire lift by a multiple gang disk with a minimum disk wheel diameter of 600 mm (24 in.).

Unless otherwise approved in writing, each embankment lift shall receive a minimum of three passes with the static roller and a minimum of two passes with the vibratory roller. The material shall be bladed before using the vibratory tamping-foot roller. A pass shall be in accordance with 402.13. The rollers shall not exceed 5 km/h (3 mph) during these passes. The number of passes will be adjusted upward if necessary to obtain 95% of maximum dry density, in accordance with AASHTO T 99 except as modified in 203.24. No additional compensation will be allowed for additional passes as specified herein, the cost of which shall be included in the costs of the pay items.

Water required to facilitate the slaking and compaction of the shale or soft rock will be measured in accordance with 203.27(h) and paid for in accordance with 203.28. No payment will be allowed for any water required for compaction of material furnished as borrow.

(c) Shale and Thinly Layered Limestone. In Dearborn, Decatur, Fayette, Franklin, Jefferson, Ohio, Ripley, Rush, Switzerland, Union, and Wayne Counties specifically, or in other area where relatively thin layered shale and rock are encountered, their use will be permitted in the construction of embankment, if the following provisions, in addition to those stated in 203.20(b), are observed.

1. The slopes shall be encased with a minimum of 3 m (10 ft) of relatively impervious, non-shale, non-erodible material.
2. The maximum size of limestone which will be permitted in the mixture shall be 150 mm (6 in.) in thickness and 0.5 m (1.5 ft) in any other dimension.
3. The minimum number of passes with static roller and the vibratory tamping-foot roller shall be six static and two vibratory.

If the material is found to be too intermixed with limestone fragments to permit field density tests as required in this section, this requirement may be waived by written permission.

203.21 Embankment on Hillsides or Slopes. Before an embankment is placed on natural soil slopes or existing fill slopes of 4:1 or flatter, the existing ground surfaces shall be plowed or deeply scarified or, if the nature of the ground indicates greater precautions should be taken for integrating the proposed fill materials with the existing slopes, benches shall be cut into the existing slopes before fill placement is started. All such precautionary work shall be done as directed. No direct payment will be made for plowing or scarifying, the cost thereof to be included in the various pay items of the contract. Before an embankment is placed on natural soil slopes or existing fill slopes steeper than 4:1, benches a minimum of 3 m (10 ft) wide, unless otherwise specified, shall be cut into the slopes prior to the placement of embankment fill. If benches are cut, the excavation involved will be paid for at the contract unit price per cubic meter (cubic yard) for the class or classes of excavation encountered.

203.22 Embankment Over Existing Roadbeds. If embankment for new pavement is to be placed over an area where a rigid pavement or any pavement having a concrete base is in place, or in other cases when required, the upper surface of which is 0.3 m (1 ft) or less below the subgrade elevation of the proposed new pavement, the existing old pavement, including any concrete base, shall be removed. The method of removal, disposal, and basis of payment shall be in accordance with 202.05 and 202.13.

If embankment for new pavement is to be placed over an area where an existing rigid pavement is in place, the upper surface of which is more than 0.3 m (1 ft) but less than 0.9 m (3 ft) below the subgrade elevation of the proposed new pavement, or in other cases when required, the existing pavement shall be broken. Pavement shall be broken so the area of any individual unbroken slab does not exceed 1 m² (1 sq yd).

If embankment for new pavement is to be placed over an area where an asphalt filled brick-type or an asphalt-type surface on a concrete base is in place, and such existing surface is more than 0.3 m (1 ft) but less than 0.9 m (3 ft) below the subgrade elevation of the proposed new pavement, or in other cases when required, the brick and cushion material, or the asphalt courses, shall be removed and the concrete base broken. Removal of the surfacing material, breaking the base, disposal of removed material, and basis of payment shall be in accordance with 202.05 and 202.13.

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If embankment for new pavement is to be placed over an area where a flexible-type pavement is in place, the top of which is at the approximate elevation of, or is 0.3 m (1 ft) or less below the required subgrade elevation of the proposed new pavement, the existing pavement shall be loosened to the depth directed, but no less than 0.3 m (1 ft). This loosened material shall be spread uniformly over the full width of the subgrade plus 0.3 m (1 ft) on each side and compacted. No direct payment will be made for this loosening, spreading, and compacting, the cost thereof to be included in the various pay items of the contract.

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If embankment for new pavement is to be placed over an existing macadam, the surface of which is more than 0.3 m (1 ft) but less than 0.9 m (3 ft) below the subgrade elevation of the proposed new pavement, the existing macadam shall be loosened to a depth sufficient to prevent possible trapping of water above the existing surface. No direct payment will be made for this loosening, the cost thereof to be included in the various pay items of the contract.

Where the existing roadbed is too narrow, except as otherwise herein provided, new pavement shall not be placed partly on old and partly on new embankment. If the fill supporting an existing roadbed is 0.3 m (1 ft) or more in depth, and is too narrow to carry the entire width of the proposed new pavement, the existing width of roadbed shall be taken down to include the new roadbed width and rebuilt from the lowest elevation of the disturbed old roadbed to the required new width. This rebuilding shall be in accordance with these specifications for constructing embankment and as directed. For the necessary tearing down of the existing embankment, payment will be made at the contract unit price per cubic meter (cubic yard) for the class or classes of excavation encountered.

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If an embankment is to be widened, due precautions shall be taken to ensure a firm foundation. After all sod and other perishable material has been removed, the existing shoulders shall be plowed down 0.6 m (2 ft) out from the existing pavement. This material shall be used for widening. Benches, a minimum of 1.2 m (4 ft) wide, shall be cut into the slope of the old embankment, unless otherwise directed. The materials from plowing down the shoulders and benching the slopes shall be deposited, spread, and compacted as set out herein for embankment, after which any remaining required embankment shall be finished with additional material, deposited and compacted in like manner. No direct payment will be made for benching, plowing, spreading, and compacting, the cost thereof to be included in the various pay items of the contract.

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203.23 Embankment Other Than Rock and Shale, With Density Control. Unless otherwise specified, all embankments shall be compacted to at least 95% of their maximum dry density. The moisture content shall be controlled within -2 and +1 percentage points of optimum moisture content. Maximum density and optimum moisture content shall be determined in accordance with AASHTO T 99, as modified in 203.24, using method A for soil and method C for granular materials.

780 If the embankment material is too wet or too dry, either the material shall be aerated to remove excess moisture or watered and disked to increase the moisture content, until in either case the moisture content is within the specified range. Sufficient moisture tests will be made to ensure that this range is maintained throughout the embankment.

The embankment material shall be placed in uniform level layers, left properly shaped as set out above, and compacted with approved compacting equipment. Compacting equipment shall include at least one three-wheel roller or other approved compacting equipment capable of providing a smooth and even surface on the embankment as directed.

790 Each lift shall be disked or treated by some other mechanical means which shall ensure the breaking up of any existing lumps and clods.

The loose depth of each lift shall be such that the required compaction can be obtained, but in no case shall it exceed 200 mm (8 in.). Where a tamping roller is used, the loose depth of lift shall not exceed the length of the tamper feet. The surface area of the end of each foot of the tamping roller shall be no less than 3550 mm² (5 1/2 sq in.).

800 When silts or loessial type soils are encountered and used in embankment construction, the moisture content shall be controlled within -3 percentage points of optimum moisture content and optimum moisture content. In no case shall these soils be placed at moisture content in excess of optimum moisture content.

The moisture content for sand soil or a sand and gravel soil, having at least 80% sand and gravel size particles, shall be such that the soils may be compacted to the specified density at a moisture content which is normally several percentage points below optimum or as directed.

810 **203.24 Method of Making Density Tests.** The percent of compaction shall be based on maximum dry densities unless otherwise specified or directed. Field compaction tests will be run on each lift and the required compaction obtained on each lift before the next is placed.

(a) Laboratory. The procedure for determining maximum densities for compaction control shall be in accordance with AASHTO T 99.

(b) Field. The field density determination shall be made in accordance with AASHTO T 191, T 205, or T 272 except as follows:

820 1. If T 191 is used, the sand used for the test shall be Silica Sand in accordance with the gradation as follows:

Passing the 850 µm (No. 20 sieve) 98% to 100%

Passing the 425 µm (No. 40 sieve) 0 to 35%

Passing the 212 µm (No. 70 sieve) 0 to 2%

Sand such as Wedron Silica Sand No. 4075 or Ottawa 2.8 Blasting Sand has been found to be acceptable.

830 2. If particles larger than those that can pass through a 4.75 mm (No. 4) sieve for soil and a 19.0 mm (3/4 in.) sieve for granular material are encountered, corrections shall be made so that the density obtained is for the minus 4.75 mm (No. 4) or 19.0 mm (3/4 in.) only. After the densities are determined, the degree of compaction shall be computed by the following formula:

$$\text{Degree of compaction} = \frac{\text{In Place Density kg/m}^3 \text{ (pcf)}}{\text{Maximum Density kg/m}^3 \text{ (pcf)}} \times 100$$

840 3. Other approved types of field density tests may be used for control purposes after density values corresponding to those obtained by either of the methods set out above have been established.

4. All references to soils in these methods of tests shall be interpreted to mean either or both soil and granular materials.

850 **203.25 Embankment Without Density Control.** When aggregate is used for embankment construction and has such a large top size as to make it impractical to perform density tests, and if approved, such material may be compacted with crawler-tread equipment or with approved vibratory equipment, or both. The materials shall be placed in lifts not to exceed 150 mm (6 in.), loose measurement, and each lift compacted thoroughly by successive trips back and forth with the tread areas overlapping enough on each trip so that all portions will be compacted uniformly.

At locations inaccessible to the above compacting equipment, the required compaction shall be obtained with approved mechanical tamps or vibrators, in which case the depth of lifts, loose measurement, shall not exceed 100 mm (4 in.).

860 **203.26 Proofrolling.** When proofrolling is specified, the work shall be performed with a pneumatic tire roller in accordance with 408.03(d)3, except the minimum tire size shall be 9:00 by 20. There shall be one or two complete coverages as directed. Roller marks, irregularities, or failures shall be corrected.

203.27 Method of Measurement

(a) Contract Quantity. The quantities of excavation for which payment will be made will be those shown in the Schedule of Pay Items for the pay items, provided the project is constructed to the lines and grades shown on the plans or revised contract quantities as adjusted by authorized change.

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Unless otherwise specified, the project limits will be considered as one balance. If earthwork balances are shown on the plans, they are for information only.

When the plans have been altered or when disagreement exists as to the accuracy of the plan quantities in any balance, or the contract quantity, either party shall have the right to request and cause the quantities involved to be measured in accordance with measured quantities. When the quantities are measured for payment, the original plan cross sections plotted on the plans shall be used as original field cross sections. Additional original cross sections may be interpolated at points where necessary to determine the quantities more accurately. If the Contractor has acceptable engineered data that indicates an excavation quantity that is in error by more than 2%, then additional measurements will be performed on the areas in question and payment will be made for actual quantities.

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(b) Measured Quantities. When payment is specified on a volume basis, all accepted excavation and borrow will be measured in its original position by cross sectioning the area excavated, which measurements will include over-breakage or slides in common excavation and unclassified excavation, not attributable to carelessness, and authorized excavation of rock, shale, peat, or other unsuitable material. Volumes will be computed from cross section measurements by the average end area method.

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Measurement for payment will not include material excavated beyond authorized cross sections. Where material is excavated beyond authorized cross sections and wasted without authority, the material so wasted will be measured and deducted from the excavation quantities. Unless otherwise authorized, the amount of waste to be deducted, when common excavation, rock excavation, unclassified excavation, borrow, or other excavation has been wasted along embankments or elsewhere without authority shall be that portion of the embankment or fill which is outside a 1/4 to 1 slope in excess of that shown on the plans, and all portions outside a line from the shoulder point to a point 1.2 m (4 ft), measured horizontally, outside the theoretical toe of the slope. In determining waste, no tolerance in widths of shoulders will be allowed unless additional widths are authorized in writing before shoulders are finally constructed.

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Measurements will be made for unsuitable materials actually excavated and removed to obtain proper compaction in cut sections and in foundations for fill sections.

Where it is impracticable to measure material by the cross section method due to the erratic location of isolated deposits, acceptable methods involving three-dimensional measurements may be used to measure the material in its original position.

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The cubic meters (cubic yards) of peat excavated will be determined by cross sections, as described above, with the final cross sections taken after complete excavation and before placing granular backfill, if peat is removed by excavation. When removal by displacement is necessary, final cross sections will be derived from cased test holes through the completed granular treatment in accordance with 203.17. Such test holes shall be located at intervals which clearly define the bottom of the treatment between lateral limits. Pay quantities of peat excavation will be limited to the volume of peat lying between vertical lines as shown on the plans for lateral limits.

920 Cased test holes and exploratory drilling will be measured by the meter (linear foot); B borrow by the cubic meter (cubic yard), in accordance with 211.09. However, measurement to neat lines will not apply unless specifically designated. Breaking pavement will be measured by the square meter (square yard) in place before breaking.

(c) Measurement on a Linear Basis. When either or both the plans and contract provide for a pay item for linear grading, the measurement for payment will be based on the length of roadway actually constructed to the lines, grades, and typical cross sections specified. All classes of excavation involved, including required borrow, 930 will not be measured for payment, but will be considered as included in the pay item for linear grading per kilometer (mile).

(d) Measurement of Excavation Items on a Weight Basis. A pay item for excavation may be specified to be measured and paid for on a weight basis. When a weight basis is specified, the material will be weighed in accordance with 109.01(b).

(e) Measurement of Embankments. When specified, embankments constructed will be measured in accordance with the terms set forth.

940 When embankment is specified as a separate pay item, the volume so constructed will be measured in place in accordance with 203.27(b). However, no measurement will be made for excavation or borrow, except as noted herein. The volume shall be computed in cubic meters (cubic yards) from the dimensions of the embankment cross sections and to the depth below completed grade to which this method of construction applies. No shrinkage factor shall be used in computing the embankment volume. Measurements will be made for unsuitable materials actually excavated and removed to obtain proper compaction in cut sections and in foundations of fill sections in accordance with 203.27(b). B borrow from off the right-of-way placed within embankment areas will be measured in accordance with 211.09.

950 **(f) Measurement of Borrow.** Borrow will be measured by the cubic meter (cubic yard). Except as otherwise provided herein, borrow will be cross sectioned in its original position before excavation is started, again after it is completed, and the volume computed by the average end area method. No material shall be excavated as borrow until unsuitable material, vegetation, and other perishable matter have been removed and cross sections taken over the cleared area. None of this removed unacceptable material shall be moved back into the pit before final sections are taken.

960 If borrow is obtained from a source where it is impracticable to measure the material in its original position, such material will be measured after being placed in embankment with no allowance made for a compaction factor. The borrow may be measured in truck beds in accordance with 211.09 for contracts having a proposal quantity less than 382 m³ (500 cu yd). If such a source is contemplated, approval shall be obtained in writing before this part of the work is started. For quantities less than 3,820 m³ (5,000 cu yd), the material may be weighed in accordance with 109.01(b) and converted from kilograms (pounds) shown on the weigh tickets to cubic meters (cubic yards) using a factor of 1780 kg/m³ (3,000 lb/yd³).

970 If borrow is specified outside the limits of B borrow and if the requirements of the work do not otherwise prohibit, material in accordance with these specifications for B borrow, may be furnished and placed at the contract unit price for borrow, in which case measurement will be in accordance with 211.09. Measurement of borrow in accordance with 211.09 will be limited to nominal quantities outside the limits of structure backfill. The quantity of borrow measured for payment will not exceed the theoretical quantity of B borrow furnished.

980 **(g) Measurement of Subgrade Treatment.** The work of stabilizing subgrade treatment areas will be measured by the square meter (square yard) or hectare (acre) from the limits of the area so treated.

(h) Measurement of Water for Shale, Shale and Soft Rock Mixtures, or Soft Rock. When payment for water for shale is specified in the contract, the water for shale used will be measured by the kiloliter (1,000 gal.) by means of calibrated tanks or distributors, or by means of accurate water meters. When water for shale is not specified as a pay item in the contract, the water for shale that is used will not be measured directly. Such measurement will be included in that required for other pay items.

990 **(i) Lump Sum.** If the pay unit for a pay item for excavation is lump sum, no measurement will be made.

(j) Measurement of Exploratory Cores. Exploratory cores will be measured by the meter (linear foot) of rock core.

1000 **203.28 Basis of Payment.** The accepted quantities of excavation and embankment will be paid for at the contract price per unit of measurement for each of the pay items listed below which is included in the Schedule of Pay Items. Common excavation and unclassified excavation will be paid for on the basis of contract quantities in accordance with 203.27(a), without any quantity limit, unless otherwise shown in the Schedule of Pay Items. Combined quantities of borrow, common and unclassified excavation not exceeding 12 000 m³ (15,000 cys) will be paid for on the basis of contract quantities in accordance with 203.27(a) with no adjustment to plan shrinkage factor. Except as noted above borrow, as well as all other excavation, will be paid for on the basis of measured quantities in accordance with 203.27(b), unless otherwise shown in the Schedule of Pay Items.

Excavation for the removal of unsuitable material will be paid for at the contract unit price per cubic meter (cubic yard) for embankment, unless otherwise specified.

1010 If the class of excavation is linear grading, additional borrow and the excavation of and disposal of unsuitable material not included as pay items will be paid for as follows:

If the total quantity exceeds 3800 m³ (5,000 cu yd) at a given location, it will be paid for at \$5.23 per m³ (\$4.00 per cu yd). If the total quantity exceeds 765 m³ (1,000 cu yd), but does not exceed 3800 m³ (5,000 cu yd) at a given location, it will be paid for at \$7.85 per m³ (\$6.00 per cu yd). If the total quantity does not exceed 765 m³ (1,000 cu yd) at a given location, it will be paid for at \$10.46 per m³ (\$8.00 per cu yd).

1020 Linear grading includes only such grading within the construction limits. All grading the Contractor is directed to perform outside the construction limits, except for the Contractor's convenience, will be paid for in accordance with 104.03 or 109.03 unless such grading is shown on the plans or in the Contract Information book.

Existing concrete building foundations, concrete walls, concrete columns, or concrete steps not visible and not shown on the plans within the limits of the planned excavation will be paid for at three times the contract unit price per cubic meter (cubic yard) for common or unclassified excavation, whichever is set out as a pay item.

1030 If it is directed that an area be excavated for the purpose of removal of unsuitable material, such excavation and disposal will be paid for at the contract unit price for the class of excavation involved.

If there is no pay item for rock excavation and such is encountered, it will be paid for at three times the contract unit price per cubic meter (cubic yard) for common excavation.

If there is no pay item for common excavation and if such is encountered, it will be paid for at the contract unit price per cubic meter (cubic yard) for borrow.

1040 If the contract includes a pay item for waterway excavation, and if class Y excavation is encountered and there is no pay item for such, the class Y excavation will be paid for at three times the contract unit price per cubic meter (cubic yard) for waterway excavation.

If the contract does not include a pay item for waterway excavation and such is encountered, such excavation within the right-of-way limits will be paid for at the contract unit price per cubic meter (cubic yard) for common excavation. If there is no pay item for common excavation, the pay item for unclassified excavation will be used.

1050

If excavation is necessary to investigate or to seal sinkholes, or to explore underground drainage, the accepted quantity involved at each location will be paid for as follows. The first 8 m³ (10 cu yd) or fraction thereof will be paid for at five times the contract unit price for the class of excavation encountered. The next 30 m³ (40 cu yd) or fraction thereof will be paid for at three times the contract unit price for the class of excavation involved. Additional quantities will be paid for at the contract unit price per cubic meter (cubic yard) for the class of excavation involved.

1060 Material overlying the peat deposits which is excavated and used in embankment will be considered as common excavation and will be paid for as such. Excavation for standard side ditches or other side ditches which are constructed through peat areas at locations shown on the plans, or where directed, will be paid for at the contract unit price per cubic meter (cubic yard) for common excavation.

Cased test holes and exploratory drilling will be paid for at the contract unit price per meter (linear foot).

Payment will be made under:

1070	Pay Item	Metric Pay Unit Symbol (English Pay Unit Symbol)
	Borrow	m3 (CYS)
	Breaking Pavement	m2 (SYS)
	Cased Test Holes.....	m (LFT)
	Embankment	m3 (CYS)
	Excavation, Common	m3 (CYS)
	Excavation, Peat	m3 (CYS)
	Excavation, Rock	m3 (CYS)
	Excavation, Unclassified	m3 (CYS)
1080	Excavation, Waterway.....	m3 (CYS)
	Excavation, Y.....	m3 (CYS)
	Exploratory Cores	m (LFT)
	Exploratory Drilling.....	m (LFT)
	Linear Grading	STA
	Subgrade Treatment.....	m2 (SYS)
	Water for Shale.....	kL (kGAL.)

1090 If embankment is specified as a pay item, borrow and common excavation, unless otherwise specified, will not be paid for directly. The costs thereof shall be included in the cost of embankment. Such price shall be full compensation for preparation of the natural ground on which the embankment is to be placed, removal and satisfactory disposal of unsuitable materials; and excavating, hauling, placing, spreading, and compaction of materials in accordance with 203.23. The costs of labor, equipment, tools, and necessary incidentals shall be included in the cost of

embankment. The cubic meterage (cubic yardage) of suitable material used in the embankment excavated from the right-of-way and paid for under a specific pay item will not be deducted from the embankment quantities. The quantity of material to be paid for as B borrow and placed within the embankment area as specified will be deducted from the quantity of embankment.

1100

The costs of excavating, backfilling, disposal of surplus material, labor, equipment, tools, and necessary incidentals necessary shall be included in the cost of excavation required to seal sinkholes or explore underground damage.

The costs of all excavated or displaced peat, regardless of depth, peat disposal, temporary surcharge, machine operation, and machine availability shall be included in the cost of peat excavation. However, the Department may provide temporary right-of-way for peat disposal when so specified.

1110

The costs of boring the holes, casings and fittings, labor, equipment, tools, and all necessary incidentals shall be included in the cost of cased test holes or exploratory drilling.

The cost of reshaping scoured or eroded areas shall be included in the costs of other pay items.

The costs of surface roughening shall be included in the costs of other pay items.

1120

The costs of identification of borrow pits, archeological investigations, and changes to construction operations caused by the identification of an archeological site shall be included in the cost of borrow, unless otherwise agreed to in writing.

SECTION 204 -- GEOTECHNICAL INSTRUMENTATION

204.01 Description. This work shall consist of providing, installing, maintaining and reading various types of geotechnical instrumentation at locations shown on the plans or as directed.

MATERIALS

204.02 Materials. Materials shall be in accordance with the following.

10

B Borrow 211.02(b)

CONSTRUCTION REQUIREMENTS

204.03 Settlement Plates. Prior to the beginning of embankment operations, 13 mm by 0.9 m by 0.9 m (1/2 in. by 3 ft by 3 ft) steel settlement plates equipped with sections of 50 mm (2 in.) pipe shall be installed at locations as shown on the plans.

20 **(a) Installation of Settlement Plates.** Each settlement plate shall be buried on a horizontal plane and on a compacted leveling layer of B borrow, not less than (0.3 m) (1 ft) below the elevation of the adjacent area. The first section of pipe shall then be installed, the hole backfilled with B borrow and thoroughly compacted over the plate and around the pipe before starting the first lift.

30 The pipe sections for the settlement plates shall be 50 mm (2 in.) steel pipe, 1.2 m (4 ft) long and threaded on both ends with proper fittings so that such pipe sections can be extended vertically from the center of the plates up through the new embankment as it increases in height during grading operations. The pipe sections shall be spot welded at the joints. The 50 mm (2 in.) steel pipes shall extend 0.6 m (2 ft) or more above the grade of the new embankments at all times during grading operations and settlement period. The settlement plates and extension pipes shall not be disturbed nor damaged during the period of time required for checking embankment settlement.

 In addition to the settlement plates as specified above, toe stakes shall be installed at the toe of slope opposite each settlement plate, at other locations shown on the plans and as otherwise directed. The toe stakes shall be 19 mm (3/4 in.) by 1.2 m (4 ft) steel rods.

40 B borrow shall be used as embankment material around settlement plates and pipe in order to protect the plates and pipe during embankment construction. The B borrow shall be placed in accordance with all applicable requirements of 211.

 After the embankment has been constructed to subgrade elevation, shoulder stakes shall be installed at the locations shown on the plans or as otherwise directed. The shoulder stakes shall be 19 mm (3/4 in.) by 1.2 m (4 ft) steel rods.

50 **(b) Instrument Readings and Settlement Period.** During the construction of the embankment, elevation readings will be taken on all settlement plate extension pipes and toe stakes at the end of each seven day period, or more frequently if the conditions warrant. After the embankment is constructed to subgrade elevation, elevation readings will be taken on the shoulder stakes, in addition to the settlement plate extension pipes and toe stakes as set out herein.

 Unless otherwise directed, the new embankment, after being constructed to subgrade elevation, shall be allowed to settle for a period of three months.

60 If the results of the instrument readings, as set out above, indicate that the new embankment, after construction to subgrade elevation, is still in a state of settlement at the end of the third month of more than 3 mm (0.01 ft) per seven day week for any of the final four consecutive weeks, then such embankments shall be allowed additional settlement time until instrument readings indicate that settlement is 3 mm (0.01 ft) or less per seven day week for four consecutive weeks.

If the results of the instrument readings as set out above indicate that the new embankment, after construction to subgrade elevation, is in a state of settlement of 3 mm (0.01 ft) or less per seven day week for four consecutive weeks prior to the end of the settlement period, the period of settlement time may be decreased as directed.

70 If serious lateral movement or settlement develops during the construction of the embankment or within the required settlement period, the work will be suspended and corrective measures taken as directed.

All settlement plate extension pipes, shoulder and toe stakes shall be protected and shall not be disturbed nor damaged. Any pipe or stake that is damaged shall be replaced with no additional payment.

204.04 Method of Measurement. Steel settlement plates with pipe extensions will be measured by the number of units installed.

80 **204.05 Basis of Payment.** Steel settlement plates with pipe extensions will be paid for at the contract unit price per each.

If the placement of additional embankment material is directed, such material will be paid for at the contract unit price for the class of excavation involved.

Payment will be made under:

	Pay Item	Pay Unit Symbol
90	Settlement Plate	EACH

The costs of furnishing, installing, and maintaining steel settlement plates, extension pipes, spot welding pipe joints, toe stakes, shoulder stakes, settlement stakes, B borrow used as embankment material around settlement plates, constructing the new embankment as set out herein, and all necessary incidentals shall be included in the cost of settlement plate.

SECTION 205 -- TEMPORARY EROSION AND SEDIMENT CONTROL

205.01 Description. This work shall consist of furnishing, installing, maintaining, and removing temporary erosion and sediment control measures in accordance with these specifications and in reasonably close conformance with the plans or as directed. Prosecution of the work shall be in accordance with 108.03.

MATERIALS

10 **205.02 Materials.** Materials shall be in accordance with the following:

Chain Link Type Fence	910.18(b)
Coarse Aggregate, Class A, B, C, D, E, or F	904.02
Farm Field Type Fence.....	910.18(a)

	Geotextile for Silt Fence	913.20
	Geotextile Under Riprap	913.18
	Metal End Section	908.06
	Metal Pipe	908
	Plastic Corrugated Drainage Pipe.....	907.17
20	Revetment Riprap.....	616.02(b)
	Wood Stakes	914.09(b)

Straw bales shall be a minimum of 356 mm by 460 mm by 920 mm (14 in. by 18 in. by 36 in.) and shall not weigh less than 16 kg (35 lb). Bales shall be bound with wire or nylon twine.

Posts for silt fence shall be either wood, metal, or synthetic. They shall be of sufficient strength to resist damage during installation and to support applied loads. Prefabricated fence systems may be used provided they are in accordance with 913.20.

30

CONSTRUCTION REQUIREMENTS

205.03 Temporary Perimeter Protection. When perimeter protection is specified, the Contractor will be permitted to use a silt fence or straw bale barrier as shown on the plans. Drainage barrier at swale shall be installed at locations shown on the plans. Straw bales shall be in accordance with 205.03(a). Silt fence shall be in accordance with 205.03(b). Stone outlets shall be located as shown on the plans. The spacing of the outlets shall be adjusted to locate outlets at low points along the barrier line.

40

(a) Straw Bales. Straw bales shall be embedded and staked as shown on the plans. Adjacent bales shall be chinked to eliminate gaps between the bales. Bales shall be placed such that the bindings are parallel to and not in contact with the ground.

(b) Silt Fence. During all periods of shipment and storage, the geotextile shall be protected from temperatures greater than 60EC (140EF) , mud, dirt, dust, and debris. The manufacturer's recommendations shall be followed with regard to protection from direct sunlight. The geotextile will be rejected if it has defects, tears, punctures, flaws, deterioration, or damage incurred during manufacture, transportation, storage, or installation. Each roll shall be labeled or tagged to provide product identification sufficient for inventory and quality control purposes.

50

Installation of the silt fence shall be in accordance with the manufacturer's instructions, and as described elsewhere herein. Silt fence construction shall be adequate to handle stresses from sediment loading. The geotextile shall be entrenched to the depth shown on the plans. The trench shall be backfilled and the soil shall be compacted over the geotextile. The geotextile shall be spliced together only at a support post with an overlap of 150 mm (6 in.) minimum. The minimum post depth shall be as shown on the plans. The geotextile shall be securely fastened to the posts in accordance with the manufacturer's recommendations.

60

The maximum post spacing shall be as shown on the plans or as recommended by the manufacturer, whichever is lesser. The spacing of the posts shall be adjusted such that the posts are located at the low points along the fence line. Joints in fabric shall be avoided at low points along the fence line. At joints, the overlap shall be nailed or similarly fastened to the nearest post with a lath.

If wire support fence is used, the wire mesh shall be fastened securely to the upslope side of the post. The wire mesh shall be entrenched as shown on the plans.

70

The Contractor shall provide the Department with original copies of all necessary current manufacturer's installation manuals and shop drawings prior to beginning installation of the silt fence. Shop drawings shall be a minimum of 550 mm by 860 mm (22 in. by 34 in.) in size. No installation work shall begin prior to the Department's receipt of such manuals and drawings. The manuals and drawings will remain the property of the Department. In case of dispute regarding installation, the manufacturer shall provide a properly trained representative, directly employed by the manufacturer, to ensure that the installation is performed in accordance with the manufacturer's recommendations.

80

205.04 Temporary Slope Protection. Exposed cut and fill slopes shall be protected from runoff with temporary interceptor ditches and temporary slope drains as shown on the plans or as directed. Fill slopes shall be roughened as slopes are graded in accordance with 203.09.

Interceptor ditches shall be graded to drain to designated slope drains or outfalls. Interceptor ditches shall be constructed to the dimensions shown on the plans and compacted with a minimum of two passes of a rubber tired vehicle, preferably a grader wheel. Interceptor ditches shall be maintained and reconstructed after each day's embankment operations.

90

Temporary slope drains shall be located as shown on the plans or as directed. Temporary slope drains shall be lengthened as the embankment is constructed. The Contractor will be permitted to use a pipe slope drain or open slope drain, as shown on the plans, unless otherwise directed. A standard metal pipe end section shall be used at the inlet of the temporary pipe slope drain. A wood inlet, as shown on the plans, shall be used with the open slope drain. The wood inlet shall be staked as shown in the plans. Temporary slope drains shall be anchored as shown on the plans or as directed.

100

205.05 Sediment Control in Side Ditches. Temporary sediment control in side ditches shall include straw bale ditch checks, riprap ditch checks, silt fence ditch checks, or ditch sediment traps as shown on the plans. Straw bales shall be in accordance with 205.03(a). Existing pipe culverts shall be protected from sediment with silt fence or ditch sediment trap as shown on the plans. Silt fence shall be in accordance with 205.03(b).

110 Ditch checks shall be located as shown on the plans. When the plans specify straw bale ditch checks, the Contractor will have the option to use riprap or silt fence ditch checks. Revetment riprap shall be in accordance with 616. Straw bale, silt fence, and riprap ditch checks shall be wide enough such that water will flow over the ditch check instead of around.

A ditch sediment trap, as shown on the plans, shall be excavated at the locations shown on the plans or as directed. When specified or directed, a 1.8 m (6 ft) high chain link fence shall be placed around the trap.

120 **205.06 Sediment Basin.** A sediment basin shall be built at the locations shown on the plans. If an excavated basin is required, the excavated soil may be used to build the dam embankment. Embankment construction shall be in accordance with 203. Revetment riprap used for overflow protection shall be in accordance with 616. When specified or directed, a 1.8 m (6 ft) high chain link fence shall be placed around the basin.

Perforation of the vertical riser of the outlet pipe shall consist of 13 mm (1/2 in.) holes spaced 75 mm (3 in.) apart on each outside valley, on the lower half of the riser. A manufactured perforated riser may be used. A trash guard will be required when directed.

130 **205.06.1 Fish Pool and Sediment Trap.** A fish pool, sediment trap, or combination fish pool and sediment trap shall be constructed as shown on the plans. The sediment trap shall be constructed downstream of the construction area prior to construction activities which may cause siltation in the stream. At the close of construction and as directed, sediment shall be removed from the upstream side of the sediment trap and disposed of as directed.

140 When the combination fish pool and sediment trap is specified, at the close of construction the sediment trap shall be reshaped to create the fish pool as shown on the plans and described herein. After excavation of the fish pool, riprap shall be removed as needed from the sediment trap to produce the specified deflector shape. The removed riprap shall be placed on the fish pool spillway, at the corners of the deflector, and scattered in the bottom of the fish pool. Additional riprap may be required for full coverage of all areas shown on the plans.

205.07 Temporary Inlet Protection. Temporary protection of storm inlets from sediment shall be accomplished with a straw bale filter, silt fence filter, or gravel filter, as shown on the plans. Straw bales shall be in accordance with 205.03(a). Silt fence shall be in accordance with 205.03(b). Curb inlets shall be protected from sediment with sandbags when the roadway is closed to traffic.

150 **205.08 Maintenance of Erosion and Sediment Control Measures.** Sediment shall be removed when it has reached one half the height of the temporary protection measure, if not sooner. Sediment shall be disposed of as directed.

Temporary erosion and sediment control measures shall be inspected after rain activities, and as directed. The temporary protection measures shall be repaired or replaced as directed.

205.09 Removal of Temporary Erosion and Sediment Control Measures.

160 Temporary erosion and sediment control measures shall remain in place until directed. Upon removal, the Contractor shall remove and dispose of all excess silt accumulations, dress the area, and vegetate all bare areas in accordance with the contract requirements. Disposal of riprap and straw shall be as directed.

205.10 Method of Measurement.

170 Silt fence, straw bale barrier, drainage barrier at swale, interceptor ditch, slope drain, straw bale ditch check, and riprap ditch check will be measured by the meter (linear foot). Ditch checks will be measured transverse to the ditch. Ditch sediment trap and sediment basin will be measured by the cubic meter (cubic yard). Culvert pipe protection, drop inlet protection, and curb inlet protection will be measured per each unit installed. Removal of sediment will be measured by the cubic meter (cubic yard). Farm field type fence will be measured in accordance with 603.07. Revetment riprap will be measured in accordance with 616.11. Pipe will be measured in accordance with 715.11. The fish pool, sediment trap, or combination fish pool and sediment trap will be measured per each of the type specified.

205.11 Basis of Payment.

180 The accepted quantities of perimeter protection, drainage barrier at swale, interceptor ditch, slope drain, straw bale ditch check, and riprap ditch check will be paid for at the contract unit price per meter (linear foot), complete in place. Ditch sediment trap and sediment basin will be paid for at the contract unit price per cubic meter (cubic yard), complete in place. Culvert pipe protection, drop inlet protection, and curb inlet protection will be paid for at the contract unit price per each unit installed, complete in place. Farm field type fence will be paid for in accordance with 603.08. Revetment riprap will be paid for in accordance with 616.12. Pipe will be paid for in accordance with 715.12. Removal of sediment will be paid for at the contract unit price per cubic meter (cubic yard).

The fish pool, sediment trap, or combination fish pool and sediment trap will be paid for at the contract unit price per each of the type specified.

190 Payment will be made under:

	Pay Item	Pay Unit Symbol
	Fish Pool.....	EACH
	Fish Pool and Sediment Trap, Combination.....	EACH
	Sediment, Remove.....	m3 (CYS)
	Sediment Trap	EACH
	Temporary Erosion and Sediment Control, Culvert Pipe Protection.....	EACH
200	Temporary Erosion and Sediment Control, Curb Inlet Protection.....	EACH

	Temporary Erosion and Sediment Control, Ditch Sediment Trap	m3 (CYS)
	Temporary Erosion and Sediment Control, Drainage Barrier at Swale	m (LFT)
	Temporary Erosion and Sediment Control, Drop Inlet Protection.....	EACH
	Temporary Erosion and Sediment Control, Interceptor Ditch	m (LFT)
210	Temporary Erosion and Sediment Control, Perimeter Protection	m (LFT)
	Temporary Erosion and Sediment Control, Riprap Ditch Check	m (LFT)
	Temporary Erosion and Sediment Control, Sediment Basin	m3 (CYS)
	Temporary Erosion and Sediment Control, Slope Drain.....	m (LFT)
	Temporary Erosion and Sediment Control, Straw Bale Ditch Check.....	m (LFT)

220

The costs of geotextile fabric, trenching, backfilling, posts, fencing, and all necessary incidentals used for the installation of silt fence shall be included in the cost of perimeter protection. The costs of straw bales, stakes, and stone outlets used for the installation of straw bale barrier shall be included in the cost of perimeter protection. The costs of straw bales, stakes, and fencing shall be included in the cost of drainage barrier at swale.

230

Payment for temporary slope drain will include the standard metal end section or wood inlet, anchors, and all incidentals necessary to perform the work.

The Contractor will be permitted to use the riprap ditch check in lieu of the straw bale ditch check with no additional payment. The costs of riprap and geotextile fabric shall be included in the cost of riprap ditch check. The costs of straw bales and stakes shall be included in the cost of straw bale ditch check.

240

The cost of construction of the fish pool, sediment trap, or combination fish pool and sediment trap shall include construction, maintenance, final reshaping, and furnishing and installing revetment riprap. Revetment riprap will not be paid for when removed and used elsewhere on the project.

The pay item temporary erosion and sediment control, ditch sediment trap shall consist of the excavation of the trap. Riprap will be paid for by the megagram (ton). Pipe will be paid for by the meter (linear foot). Fencing will be paid for by the meter (linear foot).

The pay item temporary erosion and sediment control, sediment basin shall consist of the excavation of the basin. Riprap will be paid for by the megagram (ton). Pipe will be paid for by the meter (linear foot). Fencing will be paid for by the meter (linear foot).

250

The costs of silt fence, seeding, and mulch or erosion control blanket shall be included in the cost of culvert pipe protection.

The temporary protection measures and all damaged areas of geotextiles shall be repaired or replaced as directed. Repairs or replacements which are not due to the Contractor's operations will be paid for under the pay items as the original installation.

SECTION 206 -- STRUCTURE EXCAVATION

206.01 Description. This work shall consist of the excavation and backfill or disposal of all materials required for the construction of foundations for substructures of bridges, culverts, and retaining walls; and for the furnishing and subsequent removal of all necessary materials and equipment for and the construction of cribs, cofferdams, caissons, and similar items, together with their dewatering. The work shall be in accordance with these specifications and in reasonably close conformance with the lines, grades, and typical cross sections shown on the plans or as directed.

10

All excavation for structures below the designed slope or subgrade line as shown on the plans shall be included under this item.

Unless otherwise specified, structure excavation shall include all pumping, bailing, draining, sheeting, bracing, and incidentals required for proper execution of the work.

206.02 Class X Excavation.

20

(a) General Excavation. If one or more of the following materials is encountered within the limits of foundation excavating, such shall be defined as class X excavation.

1. solid rock, hard ledge rock, slate, hard shale, or conglomerate, any of which is actually removed by blasting or use of pneumatic or equivalent tools and which could not reasonably be removed by any other method;
2. loose stones or boulders more than 0.4 m³ (1/2 cu yd) in volume;
3. concrete, masonry, or other similar materials which are parts of an old structure not shown on the plans;

30

4. timber grillages, old foundation piling, buried logs, stumps, or similar materials which extend beyond the limits of excavation so that they must be cut off. Such obstructions shall be removed back to cofferdam limits and the portions so removed within cofferdam limits will be considered as class X.

40 Material commonly known as hardpan will not be considered as class X. If material is encountered during excavation which seems to be in accordance with that defined herein as class X, notification shall be made in writing, and ample time shall be allowed to make necessary investigations and measurements to determine the class and volume of the material in question.

50 **(b) Excavation for Foundations of Traffic Support Structures.** If class X material as defined in 206.02(a) is encountered within the limits of foundation excavation for traffic support structures, overhead sign structure foundations, or high mast lighting foundations, the foundation shall be relocated as directed. If it is determined that the foundation cannot be relocated, it shall be installed at the location as shown on the plans.

60 If class X material in accordance with 206.02(a)1 is encountered, the material shall be excavated to allow the foundation to be embedded a distance that is equal to 1/2 of the remaining depth of the foundation before the material was encountered, except for overhead sign structure and highmast lighting foundations. If the total length of the anchor bolts cannot be used, they shall be cut off. A steel plate measuring 150 mm by 150 mm by 13 mm (6 in. by 6 in. by 1/2 in.), shall be welded to the bottom of the bolts. The plate shall have a hole cut which allows the bolt to pass through it and the plate and bolt shall be completely welded together around the circumference of the bolt on both sides of the plate. No butt welding is allowed. The length of the bolts shall allow the plate to be covered by 75 mm to 100 mm (3 in. to 4 in.) of concrete at the bottom of the foundation.

Overhead sign structure foundations and highmast lighting foundations shall be excavated to allow the foundation to be embedded as shown on the plans or as directed.

70 If the class X material encountered is in accordance with the definitions 2, 3, or 4 of 206.02(a), the material shall be removed to the total depth of the foundation as shown on the plans.

206.03 Wet Excavation. Wet excavation shall be defined as that portion of foundation excavation, except class X, which is below a horizontal plane designated on the plans as the upper limit of wet excavation and above the bottom of the footing as shown on the plans. If wet excavation is a pay quantity and the elevation of the upper limit of wet excavation is not shown on the plans, an elevation of 0.3 m (1 ft) above the elevation of low water level as shown on the plans shall be used as such limit.

80 **206.04 Dry Excavation.** Dry excavation shall be defined as that portion of foundation excavation, except class X, which is above the upper limit of wet excavation.

90 **206.05 Foundation Excavation, Unclassified.** If the Schedule of Pay Items provides a pay quantity of foundation excavation, unclassified, and none for wet excavation or dry excavation, then foundation excavation, unclassified shall include all work described as wet excavation and dry excavation, regardless of whether or not water is encountered, but shall not include class X. Even though designated herein as foundation excavation, unclassified, it shall be regarded in these specifications as a class of excavation.

90 If no upper limit of foundation excavation, unclassified is shown on the plans, it shall be at the original ground line except where waterway excavation, common excavation, or other classified excavation overlaps the area of foundation excavation and is a pay item. The upper limit of foundation excavation, unclassified shall be the lower limit of the overlapping classified material.

CONSTRUCTION REQUIREMENTS

100 **206.06 General Requirements.** The drainage requirements of 203.14 shall apply to excavation for structures and, in addition, adequate outlets shall be provided as shown on the plans or as directed, if within the limits of the excavation.

 All excavations shall be adequately shored to avoid damage to the structure, its approaches, adjacent roadway, embankments, tracks, buildings, or other property.

110 Footing excavation shall, in general, conform with the outlines of footings as shown on the plans, or as revised, and shall be of sufficient size to permit construction of the footings to their full required dimensions. If an existing structure is being extended, the old footings shall be adequately protected. Boulders, logs, or other unforeseen obstacles encountered shall be removed.

 The elevations of bottoms of footings as shown on the plans shall be considered approximate. The final elevations shall depend on conditions encountered during excavation, at which time other elevations may be ordered in writing if necessary to secure satisfactory foundations.

120 Where existing foundation material would not support the contemplated load safely, the plans may require, or it may be directed that foundation piles be driven in the footing area. This piling shall be furnished, driven, and paid for in accordance with 701.

206.07 Disposal of Excavated Material. Except as otherwise herein provided, material excavated for a structure or its approaches, including any material excavated beyond the pay limits of foundation excavation or its specified or approved extensions, shall, if suitable, be used for filling around the new structure, for spandrel filling, approach embankment, regular embankment, or for any combination of these, all as specified or directed.

130 If excavated material is in accordance with 211.02 and if material of this nature is required at the structure or in its approaches, then this material shall be used as special fill and placed in accordance with applicable provisions of 211.

All suitable material resulting from class X excavation shall be broken into pieces which weigh no more than 70 kg (150 lb). It shall be placed as dumped riprap around the wings of the new structure, or used in approach or regular embankment, or spilled over embankment slopes. Unless disposal locations are specifically designated, the place of disposal may be any one, all, or any combination of these locations, as directed.

140 Surplus or unsuitable material shall not be disposed of in any manner that would obstruct or pollute the stream or otherwise impair the efficiency or appearance of the structure. If there are piers in the water, permission may be granted to waste excavated material adjacent to these piers provided such waste does not obstruct the waterway. If usable excavated material is wasted without authority, the quantity so wasted will be deducted from the quantities of common excavation, borrow, or B borrow, depending on the nature of the waste and the use to which it should have been put.

150 If it is necessary to dispose of surplus or unsuitable material outside the right-of-way, it shall be done in accordance with 201.03. No direct compensation will be paid for disposing of excavated materials, the cost thereof to be included in the various pay items of the contract.

206.08 Preparation of Foundation Surfaces. All rock or other hard material, if to be left in place as a foundation surface, shall be freed of loose material, cleaned, and cut to a firm surface. The final surface shall be level, stepped, or serrated as directed. Seams shall be cleaned and filled with concrete, cement mortar, or grout. These conditions shall prevail when the foundation masonry is placed.

160 Where the masonry is to rest on a foundation surface other than those described above, the approximate bottom of the excavation shall not be disturbed. The final removal of material to the required grade shall be done carefully just prior to placing the foundation masonry. The final surface shall be left smooth and, unless otherwise designated, be level.

Notification shall be given after final excavation of each foundation is completed. No masonry shall be placed until the depth of the excavation and the character of the foundation material have been approved.

170 Before foundations that do not require piling are poured, sufficient test borings shall be made to determine the character of the underlying material for a depth of at least 1.5 m (5 ft) below the bottom of the footing. Also, before pilings are driven in foundations requiring piling, sufficient soundings shall be made to determine the character of the underlying material for a depth of 3 m (10 ft) below the bottom of the footing.

180 Rock at the bottom of spread footings shall be proof tested. Exploratory holes of 50 mm (2 in.) diameter shall be drilled into the foundation base. Holes shall be drilled into sound rock to a depth of 1.5 m (5 ft) or as directed. Three holes shall be drilled into each foundation base. Observations shall be made at each hole as follows:

1. speed of drilling
2. drill pressure
3. dropping or clogging of drill bit
4. loss of drill water, if used
- 190 5. probing of the sides of the holes with a right angled chisel point.
The chisel shall be formed from a rod of 10 mm or 13 mm (3/8 or 1/2 in.) diameter.
6. continuity of bearing material

A professional engineer shall supervise the proof testing work. A report for each hole shall be prepared and submitted for review and approval.

200 **206.09 Cofferdams and Temporary Construction Dikes.** Cofferdam drawings shall be submitted in accordance with 105.02. They shall show the proposed method of cofferdam construction and other details left open to choice or not fully shown on the plans. Cofferdam construction shall not start before these submitted drawings are approved. Such approval shall be only for insuring the Department against inadequate and insufficient cofferdams and shall not release the Contractor from any risk or responsibility in connection with this part of the work.

210 Drawings of dikes to be used in lieu of cofferdams or to be used for access to the work shall be submitted, if such dikes are to be constructed within the waterway. Approval of such drawings will not relieve responsibilities as set out elsewhere in the contract documents and will only be given if the probability of stream pollution and stream flow restriction is minimal.

Cofferdams shall be constructed for all abutments and piers where water or unstable soil is encountered or where the soil may become unstable; excessive stream pollution or stream flow restriction might occur with other construction procedures; or if necessary to support the sides of excavated areas, embankment, adjacent buildings, tracks, or other premises. In general, they shall be carried down well below bottoms of

footings, shall be well braced, and as nearly watertight as practicable. The interior dimensions shall be sufficient to provide ample clearance to permit pile driving, the construction of forms, and clearance for pumping equipment outside the forms.
220 Cofferdams shall be constructed to protect plastic concrete against damage from a sudden rising of the stream and to prevent damage to the foundation by erosion.

No timber or bracing that would extend into substructure masonry shall be left in cofferdams except with written permission.

Cofferdams which become tilted or moved laterally during the process of sinking shall be righted or enlarged to provide ample clearance. Any necessary correction shall be made with no additional payment.

230 Except as otherwise provided herein, cofferdams shall be dewatered. Pumping from the interior of a cofferdam shall not carry away any of the concrete ingredients. No pumping will be permitted for 24 h after concrete placement, unless otherwise approved.

When conditions are encountered under which it is established that no reasonable pumping will dewater the cofferdam when every practicable effort has been made to reduce the inflow of water, or other conditions are such that a foundation seal is necessary, the construction of a concrete foundation seal may be required of such dimensions as necessary. This seal shall be constructed in accordance with 702.20(f).
240

Unless otherwise permitted, all cofferdam bracing shall be removed and all sheeting removed or cut off at least 0.6 m (2 ft) below the finished ground line, except within the low-water channel it may either be removed or cut off even with the stream bed. Removal shall be such that the finished masonry will not be marred or disturbed.

206.10 Method of Measurement. Structure excavation, except wet excavation, will be measured in cubic meters (cubic yards) in its original position below the limits of roadway excavation. Wet excavation will be the theoretical quantity in accordance with 206.11(b). When structures are to be placed in embankment sections, the natural
250 ground line, as cross sectioned, will be the uppermost level of computation unless otherwise specified or shown on the plans.

Class X material encountered during the excavation of foundations for traffic support structures will be measured to the neat lines as shown on the plans below the surface of class X material.

If the pay unit for any item of excavation in the Schedule of Pay Items is lump sum, no measurement will be made.

260 **206.11 Basis of Payment.** The accepted quantities of structure excavation will be paid for at the contract unit price per cubic meter (cubic yard) or lump sum.

Unless otherwise provided, excavation for bridges will be paid for at the contract unit price or prices per cubic meter (cubic yard) for the respective class or classes involved.

If cofferdams are specified as a pay item, they will be paid for at the contract lump sum price for cofferdams.

270 If a foundation seal is constructed, it will be paid for in accordance with 702.27.

The quantity of class X excavation to be paid for will be the cubic meterage (cubic yardage) of such material actually removed from within vertical planes defining the neat lines of the footings, except where material classified as class X excavation overlays other material which shall be excavated to cofferdam limits. Such overlying strata will be paid for to the limits of the material excavated beneath it. Unless otherwise provided, no additional payment will be made for such excavation made outside these limits.

280 Except as otherwise provided herein, if class X excavation is encountered and there is no contract unit price for class X excavation, payment will be made at a unit price per cubic meter (cubic yard) which is the lesser of two times the contract unit price for wet excavation or eight times the contract unit price for dry excavation. If such unit price exceeds the lowest among the contract unit price for class B concrete in footings, the contract unit price for class B concrete above footings, the contract unit price for class C concrete in superstructure, or \$262.00 per m³ (\$200.00 per cu yd), the payment for class X excavation will be made at the lowest contract unit price.

290 If class X excavation is encountered in foundation excavation unclassified and there is no pay item shown in the Schedule of Pay Items, payment will be made at four times the contract unit price per cubic meter (cubic yard) for foundation excavation unclassified, but not higher than the lower of the contract unit prices for class B concrete above footings or class C concrete in superstructure.

300 Except as otherwise provided, the quantity of dry excavation to be paid for will be the amount of such excavation actually removed from its original position within vertical planes which are 0.5 m (18 in.) outside the neat lines of the footings and parallel thereto. Regardless of the quantity actually removed, the quantity of wet excavation to be paid for will be the theoretical volume bounded by the bottoms of footings, the upper limit of wet excavation, and vertical planes which are 0.5 m (18 in.) outside the neat lines of footings and parallel thereto. Additional payment will not be made for such excavation outside these limits.

Where it is necessary to carry a footing or a portion of a footing deeper than its elevation shown on the plans, such additional excavation, except a portion thereof classified as class X which is carried down to a plane which is 1.2 m (4 ft) below the

310 bottom of footing as shown on the plans, will be paid for as extended dry excavation, extended wet excavation, or extended foundation excavation unclassified, at a price to be determined by multiplying the contract unit price for dry excavation, wet excavation, or foundation excavation unclassified, respectively, by the factors shown below.

- 320
1. For footings or portions thereof lowered not more than 0.3 m (1 ft), the factor will be 1.25.
 2. For footings or portions thereof lowered more than 0.3 m (1 ft) and not more than 0.6 m (2 ft), the factor will be 1.50.
 3. For footings or portions thereof lowered more than 0.6 m (2 ft) and not more than 0.9 m (3 ft), the factor will be 1.75.
 4. For footings or portions thereof lowered more than 0.9 m (3 ft) and not more than 1.2 m (4 ft), the factor will be 2.00.

For a footing lowered more than 1.2 m (4 ft) below its elevation shown on the plans, all such extended excavation below the bottom of footing elevation shown on the plans to the revised bottom of footing elevation shall be done as extra work in accordance with 104.03. Payment will be made in accordance with 109.05.

330 Foundation backfill required for traffic structure supports will be paid for at two times the contract unit price for the class of concrete used within the class X excavation cavity or for the percentage of foundation, when paid for as a lump sum, embedded in the class X excavation.

All backfill material or sub-footing material required whose source is other than structure excavation will be paid for at the contract unit price for the material being used or as extra work if no unit price has been established.

340 The cost of furnishing all materials and labor associated with proof testing of rock shall be included in the costs of other pay items.

Payment will be made under:

Pay Item	Metric Pay Unit Symbol (English Pay Unit Symbol)
Cofferdam	LS
Excavation, Dry.....	m3 (CYS)
Excavation, Foundation, Unclassified	m3 (CYS)
Excavation, Wet.....	m3 (CYS)
Excavation, X.....	m3 (CYS)

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The costs of placing and compacting of all backfill when the materials used are obtained from excavation, clearing and grubbing required and not paid for under another pay item, formation of embankments made with material from structure excavation, and disposal of all surplus or unsuitable excavation, unless otherwise specified shall be included in the costs of the pay items.

360 **(a) Culverts and Retaining Walls.** This requirement will not include pipe culverts. Except as otherwise provided herein, excavation for culverts and retaining walls will not be paid for directly. The cost thereof shall be included in the cost of the class of concrete used therein. The costs of all necessary removal and satisfactory disposal of all or part of the existing old structure unless its removal is otherwise provided for, cleaning out an old channel or constructing a new channel within the right-of-way limits and widening it to the grade of the existing or proposed new stream bed as shown on the plans or as directed, construction of all necessary curbs and cofferdams and their subsequent removal, subsoil borings or soundings below bottom of footings, dewatering, disposal of excavated materials, and all labor, equipment, tools, and necessary incidentals shall be included in the cost of this work.

370 If a culvert is lowered, relocated, or material of such nature is encountered that additional excavation is necessary over and above that required at the originally planned location, the additional excavation will not be paid for if it is 8 m³ (10 cu yd) or less. Additional excavation in excess of 8 m³ (10 cu yd) will be paid for at three times the contract unit price for the class of excavation involved.

380 **(b) Bridges.** The costs of clearing right-of-way within the project limits; constructing, dewatering, and removal of cofferdams, if not a pay item; subsoil borings or soundings below bottoms of footings; final preparation of foundation surfaces; disposal of excavated material; and all labor, equipment, tools, and incidentals necessary to the satisfactory completion of the excavation shall be included in the cost of this work.

The costs of all required shop plans; furnishing, hauling, and placing necessary materials; construction; maintenance; dewatering; removal of bracing; removal of or cutting off the sheeting; and labor, equipment, tools, and necessary incidentals shall be included in the cost of cofferdams.

If cofferdams are not specified as a pay item, and if cofferdams are necessary, their cost shall be included in the cost of excavation or the concrete requiring their use.

390 If there is no pay item for dry excavation, the cost of this part of the work shall be included in the cost of the foundation concrete or for other concrete requiring such excavation. However, where waterway excavation, common excavation, or other classified excavation overlaps the area of dry foundation excavation, no deduction will be made in the pay volume of such overlapping classified material. This exception will apply only if dry excavation is not a pay item.

Except as otherwise provided in 206.05, the cost of foundation excavation unclassified shall include all work and elements of volume, and excavation described above for wet excavation and dry excavation, regardless of whether or not water is encountered. The cost of foundation excavation unclassified shall not include the cost of class X excavation.

The classifications for necessary excavation for a cantilevered wing outside the limits of foundation excavation shall be the same as those governing the excavation inside such limits. The quantity to be paid for will be that actually removed from its original position within vertical planes 0.5 m (18 in.) outside the neat lines shown on the plans and parallel thereto and above a plane 0.3 m (1 ft) below and parallel to the bottom surface of the wing.

The classifications for necessary excavation for arch superstructures outside the limits of foundation excavation or waterway excavation shall be those governing the excavation inside the limits of foundation excavation or waterway excavation, depending on the location of the excavation for the arch superstructure. Such excavation shall be bounded by vertical planes which are 0.3 m (1 ft) outside the outside faces of the arch ring and parallel thereto, by a vertical plane passing through the intersection of the intrados of the arch ring and the original ground line and parallel to the face of the abutment, and the vertical plane bounding the foundation excavation on the side adjacent to the arch ring. The lower limit shall be a sloping plane which is 0.3 m (1 ft) below a plane connecting the intersection of the intrados of the arch ring and the original ground, and the intersection of the intrados of the arch ring with the vertical plane bounding the foundation excavation.

For U-abutments, the limits of dry excavation to be paid for will be extended to include all material removed from between the limits of the wing excavation and above the elevation of the bottom of the lowest tie beam. If the bottom of the lowest tie beam is below the upper limits of wet excavation, the limits of wet excavation and dry excavation to be paid for will be extended to include material actually and necessarily removed from between the limits of the wing excavation.

Where it is necessary to excavate outside the foundation excavation limits for superstructures or extensions other than those described above, the pay limits will be extended to include all such excavation, as determined necessary.

If a suitable sump is constructed outside the pay limits as described above, such limits will be extended to include the actual lines of the sump. The additional pay limits added for the sump will not exceed 4% of the area of the footing involved.

(c) Traffic Structure Supports. If class X material is encountered during the excavation of foundations for traffic support structures, the cost of such material shall be included in the cost of the foundation material.

Anchor bolts and reinforcing steel will be paid for as provided, for furnishing, cutting if required, and placing the materials. Payment will be made for the quantity shown in the Schedule of Pay Items. No additional payment will be made for cutting and welding the steel plate onto the ends of the anchor bolts.

SECTION 207 -- SUBGRADE

207.01 Description. This work shall consist of the construction of that part of the roadbed below the subgrade intended to receive the base or surfacing material in accordance with these specifications and in reasonably close conformance with the lines, grades, and cross sections shown on the plans or as directed.

CONSTRUCTION REQUIREMENTS

- 10 **207.02 General Requirements.** Unless otherwise provided, the first 150 mm (6 in.) below the subgrade shall be compacted to the moisture and density requirements in accordance with 207.06.

Soils with a maximum density of less than 1,602 kg/m³ (100 pcf) will not be permitted within 0.6 m (2 ft) of the subgrade elevation.

During subgrade preparation and after its completion, adequate drainage shall be provided at all times to prevent water from standing on the subgrade.

- 20 Coal encountered within 0.6 m (2 ft) of the subgrade elevation shall be excavated. Coal shall be disposed of outside the right-of-way.

- 30 Unless otherwise provided, the roadbed below the subgrade shall be constructed so that it has, as nearly as possible, uniform density throughout. In both cuts and fills it shall be rolled with a three-wheel roller weighing no less than 9.1 Mg (10 t) or with other approved compacting equipment capable of providing a smooth even subgrade surface. In areas not accessible to the roller or other equipment, the required compaction shall be obtained with mechanical tamps or vibrators. The upper 150 mm (6 in.) shall be in accordance with embankment density requirements of the contract immediately prior to placing the material thereon.

Soft, yielding, or other unsuitable material which cannot be compacted satisfactorily shall be removed if corrective measures are not effective. All rock encountered shall be removed or broken off at least 150 mm (6 in.) below the subgrade surface. Holes or depressions resulting from the removal of unsuitable material shall be filled with satisfactory material and compacted to conform with the surrounding subgrade surface.

40 Paving material shall not be placed before the subgrade is checked and approved, and at no time when the subgrade is frozen or muddy. No hauling shall be done nor equipment moved over the subgrade when its condition is such that undue distortion results, unless the subgrade is protected with adequate plank runways, mats, or other satisfactory means. If limits of the work make mechanical preparation of the subgrade impracticable, hand methods may be used.

Just prior to placing the initial course on the subgrade it shall be proof-rolled in accordance with 203.26. Roller marks, irregularities, or failures shall be corrected.

50 The condition of the subgrade as finally and acceptably prepared shall prevail at the time any paving material is placed thereon and a sufficient length kept so conditioned that paving operations will not be delayed.

207.03 Subgrade for HMA Base Courses. If subgrade fine aggregate, subbase, or other nonrigid course is to be placed directly on an earth subgrade, the requirements of 207.02 shall be met. The subgrade shall be kept prepared as herein described for at least 305 m (1000 ft) ahead of initial paving operations, distance permitting.

60 The grade and cross section of the earth subgrade as finished shall be within a tolerance of not to exceed 13 mm (1/2 in.) from a true subgrade. It is permissible to finish within this tolerance by blading or other mechanical means without the use of side forms. If these methods do not finish within this tolerance, side forms may be required.

207.04 Subgrade for Approaches. If cement concrete pavement or base is to be placed on an approach for a bridge or intersecting road, the subgrade shall be prepared in accordance with 207.05.

70 For crossovers, private drives, mail box approaches, and similar miscellaneous areas and for bridge and intersecting road approaches, if material other than cement concrete is to be placed thereon, the subgrade shall be prepared in accordance with 207.03.

207.05 Subgrade for Cement Concrete Base and Pavement. This subgrade shall first be in accordance with 207.02, after which a more nearly true elevation and cross section shall be obtained with approved equipment. Any loose material remaining on the subgrade shall be compacted or removed as may be necessary. The process of checking and correcting shall continue until the subgrade is firm and true.

80 **207.06 Special Subgrade Treatment.** When specified, the first 0.6 m (24 in.) below the pavement structure subgrade shall be compacted to at least 100% of the maximum dry density as determined in accordance with AASHTO T 99, except as modified by 203.24, on all mainline pavement and ramps.

Soils with a maximum density of less than 1,602 kg/m³ (100 pcf) will not be permitted within 0.6 m (2 ft) of the subgrade elevation.

When specified for S-lines, the pavement structure subgrade shall be compacted to at least 100% of the maximum dry density with a minimum compacted depth, as follows:

90

- (a) to 150 mm (6 in.) below the subgrade elevation in cut or fill sections, where the subgrade is less than 150 mm (6 in.) above the natural ground surface;
- (b) the entire thickness, where the subgrade is between 150 mm and 600 mm (6 in. and 24 in.) above the natural ground surface; and
- (c) the upper 600 mm (24 in.) of fill, where the subgrade is more than 600 mm (24 in.) above the natural ground surface.

100

Fine grain soils shall be placed at moisture content within -2 and + 1 percentage points of its optimum, except for silts and silty loams which shall be at a moisture content within -3 percentage points of its optimum. Moisture content for sand, or sand and gravel soils of at least 80% sand and gravel size particles shall be such that they may be compacted to the specified density, normally several percentage points below optimum or as directed. Final moisture and density determination as well as proofrolling, in accordance with 203.26, shall be made on the top 200 mm (8 in.) of the pavement structure subgrade as near the beginning of the paving operation as practical.

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In fill sections, subgrade treatment shall be accomplished for specified thickness of the pavement structure subgrade during construction of the embankment.

In cut sections on mainline and ramps where shale, shale and soft rock mixtures or soft rock are not encountered, the top 450 mm (18 in.) shall be undercut, and the bottom 150 mm (6 in.) treated as necessary to comply with these provisions without removal. Undercutting 450 mm (18 in.) may be waived and only the upper 150 mm (6 in.) shall be treated when it is determined through testing in accordance with 203.24 that the lower 450 mm (18 in.) complies with the moisture and density requirements herein. The use of a calibrated nuclear gauge in the direct transmission mode shall be considered acceptable for moisture and density testing purposes, as well as those methods specifically noted in 203.24. Payment, as set out below, will be made only for the actual material that was authorized to be excavated.

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In cut sections on mainline and ramps, where shale, shale and soft rock mixtures or soft rock are encountered, these materials shall be cut as near as possible to subgrade elevation, but not above subgrade elevation. All irregularities and holes shall be graded to provide positive drainage to the shoulder drains. Where necessary, finishing to subgrade elevation shall be accomplished using No. 11 crushed stone compacted as specified herein.

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207.07 Method of Measurement. Preparation of the subgrade will not be measured for payment unless otherwise specified. The quantities of excavation for which payment will be made is the quantity of excavation for subgrade treatment, as set out in the Schedule of Pay Items, in accordance with 203.27(b).

207.08 Basis of Payment. The accepted quantities of excavation for subgrade treatment, will be paid for at the contract unit price per cubic meter (cubic yard) for the excavation and disposal of all materials of whatever character encountered in the work to treat the subgrade in accordance with the typical section as shown on the plans, in accordance with the requirements of this specification, and other applicable requirements of 200.

Payment will be made under:

Pay Item	Metric Pay Unit Symbol (English Pay Unit Symbol)
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Excavation for Subgrade Treatment	m3 (CYS)
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Unless otherwise specified, preparation of the subgrade will not be paid for directly. The cost thereof shall be included in the costs of other pay items.

No direct payment will be made for special subgrade treatment on the bottom 150 mm (6 in.) below the undercut, or on the top 150 mm (6 in.) in those cut sections where it is determined that undercutting is not necessary. Payment for normal undercutting to comply with 203.09 will be made based on the class of excavation as set out in the Schedule of Pay Items.

SECTION 208 -- FINISHING SHOULDERS, DITCHES, AND SLOPES

208.01 Description. This work shall consist of the final shaping and dressing of shoulders, ditches, and slopes by hand or machine methods, or both, to the required smoothness in accordance with these specifications and in reasonably close conformance with the elevations and cross sections shown on the plans or as directed.

Where divided pavement is constructed, each roadway with its shoulders, ditches, and slopes will be considered a separate roadway.

CONSTRUCTION REQUIREMENTS

208.02 Finishing Shoulders. Unless otherwise provided, shoulders shall be constructed of earth or other approved material which contains no sod, weeds, sticks, roots, or other perishable matter. The inside edges shall be built up slightly above the finished surface of the adjoining pavement and compacted thoroughly with a roller weighing no less than 4.5 Mg (5 t) and with the roller wheel slightly overlapping the pavement. Rolling shall continue until there is no break between the pavement and shoulders, and until the required cross section is obtained.

If rolling is not practicable on shoulders for approach pavement or other miscellaneous areas, compaction shall be obtained with mechanical tamps, vibrators, or other satisfactory means.

Except where permission has been granted to widen shoulders to dispose of surplus excavation, the outside edges shall be parallel to the pavement edges.

30 Where cuts are widened beyond the typical cross sections shown on the plans in order to obtain additional material for constructing shoulder widths required or where fills are widened to dispose of excess excavated material, the shoulders shall be finished to the widths as finally constructed.

It may be necessary to finish shoulders after the pavement is opened to traffic. As a matter of safety to traffic, the pavement shall be kept as free as possible from shoulder material and equipment. The adjacent pavement over which traffic is being routed shall be cleaned at the close of work each day.

40 **208.03 Finishing Ditches.** Ditches shall be finished to the lines and grades shown on the plans or as otherwise laid out. The edges shall be parallel to the pavement unless it is necessary to have the gradients different from that of the pavement in order to obtain proper drainage, in which case the edges shall be as determined.

208.04 Finishing Slopes All cut and fill slopes shall be constructed to the cross sections shown on the plans or to revised sections where cuts are widened to obtain additional material or fills widened to utilize excess. Cut and fill slopes shall be finished to the degree ordinarily obtained by a blade grader, scraper, or hand shovel.

208.05 Blank.

50 **208.06 Finishing at Contract Drainage Structures.** If the contract work is for bridges or culverts or for extensions thereof, the requirements of this specification shall apply to the right-of-way within the contract structure limits, unless otherwise specified.

208.07 Method of Measurement. Finishing will not be measured for payment unless otherwise provided.

60 **208.08 Basis of Payment.** Finishing shoulders, ditches, and slopes will not be paid for directly. The cost thereof shall be included in the costs of other pay items.

SECTION 209 -- FINISHING EARTH GRADED ROADS

209.01 Description. This work shall consist of leveling, shaping, and otherwise completing an earth graded road ready for acceptance when the contract is for grading or for grading and structures, but not when the contract includes paving or surfacing.

209.02 Construction Requirements. After all grading is substantially complete and structures, if any, are finished, the roadbed and cut and fill slopes shall be shaped properly and, where necessary, compacted.

10

Shaping and compacting shall be with approved equipment supplemented with hand methods if necessary. Reasonably smooth surfaces shall be obtained and finished at least to within " 30 mm (0.1 ft) of the required profile and cross sections shown on the plans or as directed.

Stones which measure more than 150 mm (6 in.) in any dimension shall not be left within 200 mm (8 in.) of the proposed subgrade for subsequent rigid type base or pavement. However, they may be left at subgrade elevation for other types, if not otherwise shown on the plans.

20

Final trimming and cleaning shall be in accordance with 210.

209.03 Method of Measurement. Finishing earth graded roads will not be measured for payment unless otherwise provided.

209.04 Basis of Payment. Finishing earth graded roads will not be paid for directly. The cost thereof shall be included in the costs of other pay items.

SECTION 210 -- FINAL TRIMMING AND CLEANING

210.01 Description. This work shall consist of trimming and cleaning the otherwise completed highway between right-of-way lines for its entire contract length.

210.02 Construction Requirements. At the time of acceptance the following conditions shall prevail for the entire contract length and also for the full right-of-way width except as hereinafter provided.

10 Debris and rubbish shall be removed and disposed of in accordance with 201.03.

Remaining loose stones and broken masonry meeting the aggregate requirements for hand laid or grouted riprap shall be stored in neat piles on the right-of-way as directed.

Weeds, brush, and stumps shall be cut close to the ground. Disposal shall be in accordance with 201.03.

20 Cut and fill slopes made or disturbed shall be left reasonably smooth and uniform. Loose and overhanging rock shall be removed.

Floors, roadways, railings, bottom chords, shoes, and seats of bridges shall be cleaned of rubbish, sand, stone, gravel, and dirt. Waterways shall be left unobstructed. Culverts and other drainage structures shall be left clean for their entire length.

30 If the contract is for construction of a new pavement or for grading and structures only on a right-of-way acquired for divided highway construction, one roadway of which has been constructed and on which the Department has assumed normal maintenance, the provisions of this specification shall not apply to the maintained portion except to those areas of such which are disturbed by the operations.

If the contract is for construction of a portion of a divided highway on a right-of-way on which no previous construction has been done or on a divided highway right-of-way on which a previous grading-only contract has been completed, then the provisions of this specification shall apply to the entire right-of-way for the full contract length.

40 Unless otherwise set out in the special provisions for a contract which includes work for patching, widening, resurfacing, surface treating, undersealing, or for a combination of these, or for a contract through which traffic is being maintained during construction, these requirements will apply only to that portion of the right-of-way disturbed by the operations.

210.03 Method of Measurement. Final trimming and cleaning will not be measured for payment unless otherwise provided.

50 **210.04 Basis of Payment.** Final trimming and cleaning will not be paid for directly. The cost thereof shall be included in the costs of other pay items.

SECTION 211 -- B BORROW FILL AND BACKFILL

211.01 Description. This work shall consist of backfilling excavated or displaced peat deposits; filling up to designated elevations of spaces excavated for structures and not occupied by permanent work; constructing bridge approach embankment; and filling over structures and over arches between spandrel walls, all with special material as hereinafter described and in accordance with this specification.

MATERIALS

10 **211.02 Materials.** The material used for special filling shall be of acceptable quality, free from large or frozen lumps, wood, or other extraneous matter and shall be known as B borrow. It shall consist of suitable sand, gravel, crushed stone, air cooled blast furnace slag, granulated blast furnace slag, or other approved material. Unless otherwise specified, B borrow shall be in accordance with one of the following gradations.

(a) Gradation for Structure Backfill.

Sieve Sizes	NOMINAL SIZES AND PERCENTS PASSING					
	50 mm (2")	37.5 mm (1 1/2")	25.0 mm (1")	12.5 mm (1/2")	4.75 mm (No. 4)	600 µm (No. 30)
63 mm (2 1/2")	100					
50 mm (2")	90-100	100				
37.5 mm (1 1/2")	70-100	90-100	100	100		
25.0 mm (1")	55-95	70-100	85-100			
19.0 mm (3/4")	45-90	55-95	70-100			
12.5 mm (1/2")	35-85	40-90	55-95	85-100	100	100
4.75 mm (No. 4)	20-65	20-70	25-75	45-85	90-100	
2.36 mm (No. 8)	10-50	10-55	15-60	25-75	75-100	
600 µm (No. 30)	3-35	3-35	3-35	5-45	15-70	70-100
75 µm (No. 200)	0-8.0	0-8.0	0-8.0	0-8.0	0-8.0	0-8.0

- 20 **(b) Gradations for Other Types of Backfill.** The material shall contain no more than 10% passing the 75 µm (No. 200) sieve and shall be otherwise suitably graded. The use of an essentially one size material will not be permitted unless approved.

(c) Coarse Aggregate For End Bent Backfill. Coarse aggregate used for backfilling end bents on beam structures shall be No. 8 or No. 9 crushed stone or BF slag, class D or higher, in accordance with 904.02.

- 30 **(d) Flowable Mortar Substitution.** When B borrow or B borrow for structure backfill is specified, the Contractor may substitute flowable mortar in accordance with 213. However, flowable mortar shall not be placed into or through standing water, unless approved in writing.

CONSTRUCTION REQUIREMENTS

211.03 General Requirements. If B borrow or B borrow for structure backfill is obtained from borrow pits, the items of obtaining the pits, their locations, depths, drainage, and final finish shall comply with the provisions for these items in accordance with 203.

Unless otherwise specified, if material excavated under these requirements complies with this specification for B borrow or B borrow for structure backfill, and if B borrow for structure backfill is required for special filling by these specifications, or by the plans or special provisions, then such excavated material shall be used and placed as such or as directed. If there is a surplus of this material as excavated beyond any of its special filling requirements at locations where its use is required, such surplus shall be used in embankment. The provisions of 203.19 shall apply to placing this material at structures. All surplus in excess of the directed or specified use on the right-of-way shall be disposed of in accordance with 201.03.

50

If fill or backfill as described in this specification is within embankment limits, and if it is not required that the entire fill or backfill be of B borrow and placed as such, then that portion above free-water level shall be placed in accordance with applicable provisions of 203 and compacted to the required density.

If borrow is required outside the specified limits of B borrow, material in accordance with the specifications for B borrow may be furnished at the contract unit price for borrow; however, the quantity of borrow measured for payment outside the limits of structure backfill will not exceed the theoretical quantity of B borrow furnished.

60

Unless otherwise specified, all spaces excavated for and not occupied by bridge abutments and piers, if within embankment limits, shall be backfilled to the original ground line with B borrow, and placed in accordance with 211.04.

Where B borrow or B borrow for structure backfill is required as backfill at culverts, retaining walls, sewers, manholes, catch basins, and other miscellaneous structures, it shall be compacted in accordance with 211.04.

70

211.04 Mechanical Compaction. Where B borrow or B borrow for structure backfill is to be placed by mechanical compaction, it shall, unless otherwise specified, be placed in accordance with the applicable provisions of 203.23 except, if mechanical tamps or vibrators are used, the material shall be deposited in approximately 150 mm (6 in.) horizontal layers, loose measurement, and each layer compacted to density requirements.

211.05 Blank.

80

211.06 Embankment for Bridges. When special filling is required, the embankment for bridges shall be constructed using B borrow within the specified limits shown on the plans. All embankment construction details specifically set out in this specification for embankment for bridges shall be considered in accordance with the applicable requirements of 203.

At the time B borrow is being placed for approach embankment, an earth, watertight, well compacted dam shall be constructed in level lifts, the details of which are shown on the plans. Except as hereinafter specified for material to be used in constructing the enclosing dam, and for growing vegetation, and unless otherwise

provided, the material for constructing bridge approach embankment shall be B borrow compacted by mechanical methods. If approach embankment or shoulders are constructed of material not suitable for growing seed or sod, and if one or both of these is required, then such areas shall, unless otherwise specified, be covered with a layer of clay, loam, or other approved material which is suitable. This layer shall be approximately 0.3 m (1 ft) thick after being compacted into place.

211.07 B Borrow Around Bents. When specified, B borrow shall be placed around all bents falling within the limits of the approach grade as shown on the plans. Before placing, the surface of the ground on which it is to be placed shall be scarified or plowed as directed. The embankment slope shall be 2:1 on the sides and beneath the structure, and shall be 6:1 from the end of the bridge down to the average ground line, or it may be required to complete the approaches back to the existing grade. The enclosing dam and provisions for growing vegetation shall be in accordance with 211.06.

211.07.1 Aggregate For End Bent Backfill. When specified, coarse aggregate in accordance with 211.02(c) shall be placed behind end bents as shown on the plans. The material shall be deposited in lifts not to exceed 300 mm (12 in.) loose measurement, and each layer shall be mechanically compacted using a hand held vibratory plate compactor having a plate width of 425 mm (17 in.) or larger that delivers 13.3 to 40 kN (3000 to 9000 lb) per blow. Each layer shall be compacted with two passes of the compactor.

Prior to placing the aggregate, a geotextile in accordance with 913.18 shall be installed in accordance with 616.10.

211.08 Spandrel Filling. Unless otherwise specified, spandrel fills for arch structures shall be composed of B borrow. The fill shall be carried up symmetrically in horizontal layers from haunch to crown and simultaneously over all piers, abutments, and arch rings.

211.09 Method of Measurement. B borrow, B borrow for structure backfill, and aggregate for end bent backfill will be measured by the cubic meter (cubic yard) as computed from the neat line limits shown on the plans, or as adjusted. If cubic meters (cubic yards) are set out as the pay unit for B borrow or B borrow for structure backfill in the Schedule of Pay Items and if neat line limits are not specified for measurement of volume for the material, measurement will be made by the cubic meter (cubic yard) at the loading point in truck beds which have been measured, stenciled, and approved. The B borrow may be weighed and converted to cubic meters (cubic yards) by assuming the mass per cubic meter (weight per cubic foot) to be 90% of the standard maximum wet density as determined in accordance with AASHTO T 99, as modified in 203.24. The material may be cross sectioned in its original position and again after excavation is complete, and the volume computed by the average end area method. If B borrow is used for backfill in areas where unsuitable material is present or peat excavation has been performed, unless otherwise directed, the B borrow will be cross sectioned, and the volume will be computed by the average end area method.

If the material is to be paid for by the megagram (ton), it will be weighed in accordance with 109.01(b).

- 140 If the material comes from a wet source such as below water or a washing plant, and weighing is involved in the method of measurement, there shall be a 12 h drainage period prior to the weighing.

Geotextiles will be measured in accordance with 616.11.

211.10 Basis of Payment. The accepted quantities of B borrow will be paid for at the contract unit price per cubic meter (cubic yard) or per megagram (ton) as specified, complete in place.

- 150 B borrow for structure backfill will be paid for at the contract unit price per cubic meter (cubic yard), based on the neat line limits shown on the plans or as adjusted for authorized changes, provided the material comes from outside the permanent right-of-way. If the schedule of pay items does not contain a pay item for B borrow for structure backfill and it is required to backfill pipes or culverts within the project limits, a change order will be generated to establish a unit price.

B borrow material placed outside the neat lines will be paid for as borrow when such B borrow eliminates required borrow material. Otherwise, no payment will be made for backfill material placed outside the neat lines.

- 160 If material which is in accordance with the requirements for B borrow is obtained within the excavation limits of the project and is used as such, it will be paid for at the contract unit price for the class of excavation involved. No further payment will be made.

Aggregate for end bent backfill will be paid for at the contract unit price per cubic meter (cubic yard), based on the neat line limits shown on the plans or as adjusted by authorized changes.

- 170 Geotextiles will be paid for in accordance with 616.12.

Flowable mortar which is substituted for B borrow or B borrow for structure backfill will be paid for as B borrow or B borrow for structure backfill, respectively.

If topsoil, loam, or other suitable material in accordance with 211.06 is used for expediting the growth of seed or sod, it will be paid for at the contract unit price per cubic meter (cubic yard) for borrow, or for salvaged stockpiled selected materials as shown on the Schedule of Pay Items, unless otherwise provided.

180 Payment will be made under:

Pay Item	Metric Pay Unit Symbol (English Pay Unit Symbol)
Aggregate For End Bent Backfill	m3 (CYS)
B Borrow	m3 (CYS)
	Mg (TON)
B Borrow for Structure Backfill	m3 (CYS)

190 The cost of disposal of excavated material shall be included in the cost of the pay items.

SECTION 212 -- STOCKPILED SELECTED MATERIALS

212.01 Description. This work shall consist of excavating selected road material from within the construction limits and stockpiling it on the right-of-way at designated locations. It also includes any subsequent removal of the material from the stockpile, if to be used in the work.

10 **212.02 Materials.** Any material to be excavated and stockpiled will be specifically named and described in the special provisions and may include rock, top soil, material in accordance with 211.02, or any other material selected, any of which may be excavated as common excavation.

After the selected material is stockpiled it shall be known as stockpiled selected material and if any of this material is required to be removed from the stockpile and used in the work, its removal and its incorporation into the work shall be known as salvaged stockpiled selected material.

20 **212.03 Construction Requirements.** Selected material shall be excavated from specified areas and stockpiled on the right-of-way at designated locations. The depth of excavation shall be as directed.

If the material is required to be taken from the stockpile and utilized in the work, the material so utilized shall be placed in accordance with these specifications as they apply to the nature of the material and the use to which it is put.

30 **212.04 Method of Measurement.** Stockpiled selected material will be measured in the stockpiles by means of cross sections by the cubic meter (cubic yard), computed by the average end area method. The volume measured as salvaged stockpiled selected material will be the difference in cubic meters (cubic yards) between that of the existing stockpile and that remaining after the material has been removed from the stockpile and used. Both stockpiles will be measured by means of cross sections. The volume will be computed by the average end area method.

If stockpiled selected material is obtained from within the excavation pay limits of new construction, the volume of the material will be deducted from the pay quantities for excavation as shown on the original cross sections.

40 **212.05 Basis of Payment.** The accepted quantities of stockpiled selected material and salvaged stockpiled selected material will be paid for at the contract unit price per cubic meter (cubic yard), complete in place.

Payment will be made under:

Pay Item	Metric Pay Unit Symbol (English Pay Unit Symbol)
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Stockpiled Selected Material	m3 (CYS)
Stockpiled Selected Material, Salvaged.....	m3 (CYS)

50 The costs of excavation, hauling, removing material from the stockpile, placing materials, and necessary incidentals shall be included in the costs of the pay items.

SECTION 213 -- FLOWABLE MORTAR

213.01 Description. This work shall consist of placing flowable mortar to fill trenches for pipe structures, culverts, utility cuts and other work extending under pavement locations, to fill cavities beneath slopewalls and other locations in accordance with 105.03.

MATERIALS

10 **213.02 Materials.** Materials shall be in accordance with the following:

Coarse Aggregate, except steel slag.....	904.02
Concrete Admixtures.....	912.03
Fine Aggregate, except steel slag.....	904.01
Fly Ash.....	901.02
Portland Cement.....	901.01(b)
Water	913.01

20 If fly ash is used as a filler and not as a pozzolanic material, it shall be in accordance with the applicable requirements of 904.01(e).

CONSTRUCTION REQUIREMENTS

213.03 Proportioning. The Contractor shall submit a mix design and shall arrange a trial batch demonstration to ensure compliance in accordance with the requirements listed herein. The mix design shall include a list of all ingredients, the

source of all materials, the gradation of all aggregates, the names of all admixtures and dosage rates, and the batch weights. Except for adjustments to compensate for routine moisture fluctuations, mix design changes after the trial batch verification shall be documented and justified prior to implementation by the Contractor. A change in the source of materials, addition or deletion of admixtures or cementitious materials will necessitate a new mix design. However, a new mix design will not be required for a change from one Department-approved natural fine aggregate source to another Department-approved natural fine aggregate source having the same fine aggregate gradation. The Contractor may be required to provide test data from a laboratory inspected by the CCRL, and approved by the Department, which shows that the proposed mix design is in accordance with the strength limitations.

Only the materials listed in 213.02 may be used in the flowable mortar mix designs. The proposed mix design materials and proportions shall be submitted to the Department. Final proportioning will be determined based on the approved mix design.

213.04 Mix.

(a) Flow. The test for flow shall consist of filling a 75 mm (3 in.) diameter by 150 mm (6 in.) high open-ended cylinder placed on a smooth level surface to the top with the flowable mortar. If necessary, the cylinder shall be struck off so that the mixture is level. The cylinder shall be pulled straight up within 5 s. The spread of the mortar shall be measured. The diameter of the mortar spread shall be at least 200 mm (8 in.). Minor flow adjustments may be made by making minor adjustments in the water or fly ash filler content in the mixture.

(b) Average Penetration Resistance. The average penetration resistance in 14 days shall not be greater than 34 500 kPa (5,000 psi) nor less than 8000 kPa (1,200 psi) in accordance with ASTM C 403.

(c) Mixing Equipment. The mixing equipment shall be in accordance with the applicable requirements of 702 or 722, except that in lieu of the calibration requirements of 722.11, the mixer operator shall make delivery in a properly calibrated continuous mixer.

(d) Placement. The mixture shall be discharged from the mixing equipment by a reasonable means into the spaces to be filled. The flowable mortar shall be brought up uniformly to the fill line as shown on the plans or as directed. Placing of material over the flowable mortar may commence as specified herein or as directed.

The materials shall be mixed, delivered, and placed within 2 1/2 h.

70 Voids beneath reinforced concrete bridge approach pavement shall be filled as directed. Holes shall be drilled at locations as directed and in accordance with 612.04. The flowable mortar shall be placed until the bridge approach pavement has uniform support by means of completely filling all voids. During the filling operation, plugs may be required. Plugs shall be installed to confine the mortar as directed. The bridge approach pavement shall not lift off the bridge seat.

(e) Limitation of Operations. The flowable mortar shall not be placed on frozen ground. Flowable mortar shall be protected from freezing until the material has set.

80 The mortar shall not be subject to load nor disturbed by construction activities until an average penetration resistance has been achieved for a minimum of three readings of not less than 500 kPa (70 psi) for portland cement concrete pavement or 8000 kPa (1,200 psi) for HMA pavement. Penetration resistance shall be determined in accordance with ASTM C 403.

213.05 Blank.

90 **213.06 Method of Measurement.** Flowable mortar will be measured by the cubic meter (cubic yard) as computed from the neat line limits shown on the plans, or as adjusted. If neat line limits are not shown on the plans, the volume in cubic meters (cubic yards) of flowable mortar furnished and placed will be computed from the nominal volume of each batch and a count of the batches. Unused and wasted flowable mortar will be estimated and deducted. Drilled holes will be measured by the number of holes drilled.

213.07 Basis of Payment. The accepted quantities of flowable mortar will be paid for at the contract unit price per cubic meter (cubic yard) furnished and placed.

100 Filling voids beneath concrete bridge approach pavement will be paid for at the contract unit price per cubic meter (cubic yard) for flowable mortar. Holes drilled in the pavement will be paid for at the contract unit price per each.

 Payment will be made under:

Pay Item	Metric Pay Unit Symbol (English Pay Unit Symbol)
Drilled Hole for Flowable Mortar	EACH
Flowable Mortar	m3 (CYS)

110 The costs of material placed outside the neat line limits, material placed outside the adjusted limits, and unused or wasted flowable mortar shall be included in the cost of this work.

SECTION 214 -- DETENTION PONDS

214.01 Description. This work shall consist of the construction of detention ponds including excavation, and furnishing and placement of the necessary liner.

214.02 Materials. The soil used in the detention pond liner shall be in accordance with AASHTO M 145, classification A-6 or A-7. However, the soil in the liner shall be classified in accordance with 903 as clay or silty clay.

10 **214.03 Construction Requirements.** The detention pond shall be constructed to the lines and grades as shown on the plans.

The sides and bottom of the detention pond shall be lined with a soil liner of 600 mm (2 ft) minimum thickness. The soil in the liner shall be compacted to 95% of the maximum dry density in accordance with 203.23.

20 If soils in accordance with the above requirements are not available near the project site, the Contractor may use a lining system. Details of the proposed pond lining system shall be submitted to the Department's Geotechnical Section for approval. These details shall include all necessary information such as liner thickness, smooth surface versus textured surface, thickness and type of proposed soil cover, joint construction, material used in the liner, and manufacturer of the liner.

214.04 Method of Measurement. Excavation for detention ponds will be measured by the cubic meter (cubic yard) of the actual quantity removed. Liner will not be measured for payment.

30 **214.05 Basis of Payment.** The accepted quantities of excavation for detention ponds will be paid for at the contract unit price per cubic meter (cubic yard). The liner will be paid for as a lump sum.

Payment will be made under:

Pay Item	Metric Pay Unit Symbol (English Pay Unit Symbol)
Excavation for Detention Pond	m3 (CYS)
Liner for Detention Pond	LS

SECTION 215 -- CHEMICAL MODIFICATION OF SUBGRADE SOILS

215.01 Description. This work shall consist of the modification of fine grained soils by uniformly mixing dry portland cement, fly ash, lime, or a combination of the materials with soil to aid in achieving the workability of soils having an excessive moisture content.

MATERIALS

- 10 **215.02 Materials.** Materials shall be in accordance with the following:

Fly Ash.....	901.02
Lime.....	913.04(b)
Portland Cement, Type I.....	901.01(b)
Water	913.01

CONSTRUCTION REQUIREMENTS

- 20 **215.03 Testing and Mix Design.** The Contractor shall be responsible for all tests required to determine the optimum chemical modifier content for modification of the soils. The modifier selection, laboratory testing, and mix design shall be performed by an approved geotechnical consultant in accordance with criteria contained in the Department's guidelines, Soil Modification/Stabilization Utilizing Chemicals. Test results, recommendations, and material compliance certifications shall be submitted to the Materials and Tests Division for approval at least five days prior to use.

- 30 The quantity of chemical modifier may be adjusted for different soil types. However, the source or type of chemical modifier shall not be changed during the progress of the work without approval. A change in source or type shall require a new mix design.

215.04 Storage and Handling. The chemical modifier shall be stored and handled in accordance with the manufacturer's recommendations.

215.05 Weather Limitations. The chemical-soil modification shall be performed when the soil has a minimum temperature of 7EC (45EF), measured 100 mm (4 in.) below the surface of the subgrade, and with the air temperature rising. The chemical modifier shall not be mixed with frozen soils or with soil containing frost.

- 40 **215.06 Preparation Of Subgrade.** The subgrade shall be prepared in accordance with 207. All aggregates which are larger than approximately 75 mm (3 in.) encountered before or after mixing the soils and chemical modifiers shall be removed.

- 50 **215.07 Spreading of Chemical Modifiers.** The subgrade shall be scarified or disked to the specified depth prior to distribution of the chemical modifier. The chemical modifier shall be distributed uniformly over the subgrade by a cyclone, screw-type, or pressure manifold type distributor. The chemical modifier shall not be applied when wind conditions create problems in adjacent areas or create a hazard to traffic on any adjacent roadway. The spreading of the chemical modifier shall be limited to an amount which can be incorporated into the soil within the same work day. If weather causes stoppage of work or exposes the chemical modifier to washing or blowing, additional chemical modifier may be spread when the work resumes.

60 **215.08 Mixing.** The chemical modifier, soil, and water when necessary, shall be thoroughly mixed by rotary speed mixers or a disc harrow. The mixing shall continue until a homogenous layer of the required thickness has been obtained. One hundred percent of the material, exclusive of rock particles, shall pass a 25 mm (1 in.) sieve and at least 60% shall pass a 4.75 mm (No. 4) sieve. The loose thickness of a single chemical modified layer shall not exceed 200 mm (8 in.) if a disc harrow is used and 400 mm (16 in.) if a rotary speed mixer is used.

215.09 Compaction. Compaction of the mixture shall begin as soon as practicable after mixing. Compaction after mixing shall be as follows:

 (a) Cement modified soils shall be compacted within 30 min.

 (b) Fly ash modified soils shall be compacted within 4 h.

70 (c) Lime modified soils shall be compacted within three days.

 If compaction of lime modified soils is delayed, the surface of the lime modified soils shall be crown graded and primed in accordance with 405. When compaction is resumed, it shall be continued until the upper 150 mm (6 in.) of the chemical modified layer has the soil density determined by mix design.

 Maximum dry densities will be determined in accordance with AASHTO T 272 at the same time and location as each in-place density test is performed. The field in-place dry density shall be in accordance with AASHTO T 191 or AASHTO T 238.

80 Aeration or drying by further mixing, or the addition of water and further mixing, may be required to obtain the optimum moisture in order to achieve the required compaction. The final compaction acceptance will be in accordance with 203.26. The Contractor is fully responsible for achieving the required compaction.

215.10 Method of Measurement. The accepted quantity of modified subgrade will be measured by the cubic meter (cubic yard), complete in place. All excavation required to modify the soils below the specified depth will be measured in accordance with 203.27(b).

90 **215.11 Basis of Payment.** The accepted quantity of modified subgrade will be paid for by the cubic meter (cubic yard), complete in place. All excavation required to modify the soils below the specified depth will be paid for in accordance with 203.28.

 Payment will be made under:

Pay Item	Metric Pay Unit Symbol (English Pay Unit Symbol)
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Subgrade, Modified	m3 (CYS)
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100 The costs of performing the laboratory tests, providing an approved geotechnical consultant, scarification of the subgrade, spreading and mixing of the chemical modifier and soil, priming of the surface of the chemically modified soils, compaction of the resultant mixture, shaping the subgrade, work required due to adjustments of modifier proportioning, additional modification required due to weather conditions, correction of deficient areas, water required for the modification process, modified subgrade trimming, and all other materials and operations needed to meet the requirements of this specification shall be included in the costs of the pay item.

SECTION 216 –CELLULAR CONCRETE FILL, CCF

216.01 Description. This work shall consist of furnishing and placing a light weight, low absorbability cellular concrete fill in accordance with 105.03.

MATERIALS

216.02 Materials. Materials shall be in accordance with the following:

10	Cement	901.01(b)
	Fly Ash.....	901.02
	Water	913.01

An admixture in accordance with 912.03 may be used as recommended by the CCF manufacturer.

20 A foam liquid concentrate in accordance with ASTM C 796 shall be used to produce the CCF properties in accordance with 216.04. The concentrate shall be chosen from those shown on the Department’s list of approved CCF Manufacturers/Installers.

CONSTRUCTION REQUIREMENTS

216.03 Mix Design. A mix design prepared in accordance with the geotechnical report shall be submitted to the Materials and Tests Division for approval at least five work days before the CCF operations begin. A cellular concrete manufacturer shall be chosen from those shown on the Department’s list of approved CCF Manufacturers/Installers.

30 **216.04 Properties and Tests.** The CCF shall be in accordance with the manufacturer's recommendations and the minimum physical properties as follows:

PROPERTIES	CLASS II	CLASS III	CLASS IV
Cast Density Mix	480 kg/m ³ (810 lb/cyd)	580 kg/m ³ (972 lb/cyd)	670 kg/m ³ (1134 lb/cyd)
Minimum Compressive Strength, ASTM C 495 *	375 kPa (40 psi)	550 kPa (80 psi)	825 kPa (120 psi)
Freeze-thaw resistance minimum at relative E = 70%, ASTM C 666 Modified		80 Cycles	300 Cycles
Water Absorption Maximum **	20%	16%	14%
Shear Modulus, G, ASTM D 4015	172,000 kPa (25,000 psi)	231,000 kPa (34,000 psi)	
Young Modulus, E, ASTM D 4015	469,000 kPa (67,000 psi)	772,000 kPa (102,000 psi)	

* Specimens shall not be oven dried for the compressive strength test.

** Percentage after 120 days. Long term total immersion as a percent of cast density in accordance with ASTM C 796.

(a) CCF Cast Density. The density shall be monitored at the point of placement at hourly intervals during placement. Adjustments shall be made as necessary to maintain the specified cast density, $\pm 10\%$. If two consecutive test results are failing, operations shall cease and corrective action taken before placement of the CCF resumes.

(b) Foam Density. Foam density shall be tested twice during each 24 h period for each mixer. The foam density shall be within 10% of the target provided in the approved mix design.

216.05 Storage and Handling. Protection of the material during and after placement shall be in accordance with the manufacturer's recommendations.

216.06 Weather Limitations. CCF shall be placed when the ambient temperature is 0EC (32EF) or above. CCF shall not be placed on frozen subsoil. The installation procedure shall not begin if a temperature of less than 0EC (32EF) is expected within a 10 h time period from the completion of the CCF placement, unless recommended by the manufacturer.

216.07 Preparation of Subgrade. The subgrade shall be prepared in accordance with 207. All standing water shall be removed prior to placement of CCF.

216.08 Installation. CCF shall be proportioned, mixed, and placed in lifts as recommended by the manufacturer. Transit mixers will not be acceptable for mixing the CCF. The CCF shall not be subjected to load nor disturbed by construction activities until a minimum compressive strength of 175 kPa (25 psi) has been achieved.

The final surface finish shall be within ± 30 mm (0.1 ft) of the plan elevation.

216.09 Lots. Lots will be defined as 250 m³ (300 cyd) of CCF placed. A partial lot equal to or less than 50 m³ (60 cyd) shall be included in the previous lot. A partial lot greater than 50 m³ (60 cyd) but less than 250 m³ (300 cyd), will be considered a full lot.

70

216.10 Testing. Acceptance of the work will be based on successful test results for compressive strength. The Contractor shall cast four specimens for each lot. Testing of the specimens will be in accordance with ASTM C 495, except test specimens shall be covered immediately after casting. The specimens shall be moist cured for 26 days and then air cured for two days prior to the compressive strength testing. The specimens shall not be oven dried. Test specimens will become the property of the Department after curing and will be tested at 28 days.

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216.11 Method of Measurement. CCF will be measured by the cubic meter (cubic yard) for each class as computed from the neat line limits shown on the plans, or as adjusted.

216.12 Basis of Payment. CCF will be paid for at the contract unit price per cubic meter (cubic yard) of the class specified.

Payment will be made under:

90

Pay Item	Metric Pay Unit Symbol (English Pay Unit Symbol)
Cellular Concrete Fill _____ class	m3 (CYS)

Water, curing, molds, equipment, materials, and other incidentals necessary for finishing CCF specimens shall be included in the cost of CCF.

No payment will be made for replacement of damaged CCF.